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**Cabot WaterWorks  
Standard Construction Specifications  
Water & Wastewater Facilities**

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## **Cabot WaterWorks Standard Construction Specifications**

**Definitions of terms:** Whenever used in these specifications or the contract document the following terms shall have meanings as follows:

- (a) ADEQ: Arkansas Department of Environmental Quality
- (b) AHTD: Arkansas State Highway and Transportation Department
- (c) CWW: **Cabot WaterWorks**
- (d) Engineer: CWW' authorized Engineer, either acting directly or through field engineers or inspectors, such field engineers or inspectors acting within the scope of the particular duties assigned to them.
- (e) CONTRACTOR: Firm, Corporation or Developer executing the contract with CWW for the performance of the work hereunder.
- (f) Superintendent or Foreman: Representative of CONTRACTOR, authorized to receive and fulfill instructions from Engineer.
- (g) Contract: The contract consists of the instructions to bidders, the proposal, the contract agreement, the specifications, plans, supplemental specifications and other supplemental agreements all as required for the full execution and satisfactory completion of the work.
- (h) Plans: The approved maps, standard drawings, supplemental drawings and sketches which show the location, character, dimensions and details of the work to be done. CONTRACTOR will be supplied with copies of the plans and specifications. Any alterations affecting the requirements and information given in the plans and specifications shall be authorized in writing.
- (i) Specifications: The directions, provisions and requirements contained herein and referred to here in pertaining to the work.
- (j) Supplemental Specifications: Specific instructions setting forth conditions or requirements peculiar to the project under consideration not completely covered by the specifications.
- (k) Supplemental Agreements: The written agreements executed by CONTRACTOR and CWW covering alterations in the contract, unforeseen work and materials incident and necessary to the project.
- (l) The Work: All work specified herein and as indicated on the various plans as may be furnished to CONTRACTOR from time to time.
- (m) Title: The title to all work completed and in the course of construction at the site and of all materials furnished by CWW, irrespective of the location thereof as between CWW and CONTRACTOR or subcontractors, shall be CWW.
- (n) Authority of Engineer: The Engineer shall decide any and all questions which may arise as to the quality and acceptability of work performed and the rate of progress of the work and shall decide all questions that may arise as to the interpretation of the plans and specifications and all questions as to the acceptable fulfillment of the terms of the contract.
- (o) Assistants: The Engineer may appoint such assistants as he desires. They shall have the authority to give instructions pertaining to the work, to approve or reject work, to make measurements of quantities, to keep records of cost, and otherwise represent the Engineer. The assistants shall not be authorized to revoke, alter, enlarge, relax or release any requirement of the contract agreement, plans and specifications nor shall they act as foreman for the CONTRACTOR.
- (p) Undeveloped Areas: New subdivisions and private or public rights-of-way where other utilities facilities are not present and where traffic maintenance is not major concern.
- (q) Developed Areas: Old subdivisions and other developed areas along private or public rights-of-way where other utilities' facilities are present and do cross the path of CWW' proposed pipeline route and where traffic maintenance is of major concern.
- (r) Test Hole: Excavation made at Engineer's request prior to start of project for the purpose of determining location of existing facilities to aid in choice of alignment and grade for pipelines. To include excavation and tamped backfilling of hole approximately 4 feet wide x 4 feet long x 6 feet deep and reseeding or sodding surface. Removal and replacement of paving not to be included.

### **1. STORM WATER POLLUTION PREVENTION PLAN (SWPPP)**

- 1.1 If the Contractor disturbs more than one acre of land area through clearing, grading or excavating activities, then the Contractor shall prepare a Storm Water Pollution Prevention Plan (SWPPP) in strict accordance with the Arkansas Department of Environmental Quality and shall be approved by Cabot Waterworks.
- 1.2 If the Contractor disturbs less than one acre of land that is part of a larger development, then the Contractor shall prepare a Storm Water Pollution Prevention Plan (SWPPP) in strict accordance with the Arkansas Department of Environmental Quality and shall be approved by Cabot Waterworks.
- 1.3 The SWPPP shall be prepared and implemented prior to construction activity in strict accordance with ADEQ requirements.

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- 1.4 The SWPPP shall be maintained throughout the duration of the project in strict accordance with ADEQ requirements.

### **2. PROTECTION OF VEGETATION**

- 2.1 The CONTRACTOR shall not remove or disturb any vegetation except that required for execution of the work.
- 2.2 Unless otherwise directed in these specifications or Plans, the CONTRACTOR shall replace all sod, shrubbery, trees, and flowers disturbed or removed, that are located upon public and private property. The CONTRACTOR shall replace vegetation with healthy vegetation of the same kind or type and re-landscape or cause such to be performed throughout the work area as soon as possible after the pipelines and appurtenances have been installed. All vegetation shall be replanted in the original location. The CONTRACTOR shall maintain all replanted vegetation by the application of water, fertilizers and top soil. The vegetation shall be cultivated to prohibit the growth of foreign vegetation until a well developed root system has been established and transplanted vegetation has overcome the "shock" resulting from transplanting. All vegetation which dies or becomes unhealthy shall be replaced by the CONTRACTOR. The contour of the ground shall be left as near the original contour as possible.
- 2.3 The CONTRACTOR shall restore the topography of the work site to facilitate proper drainage of all adjacent lands.
- 2.4 The CONTRACTOR shall stabilize all areas where ground surface has been disturbed and erosion is likely to occur in accordance with the approved Storm Water Pollution Prevention Plan (SWPPP). The method of stabilization shall be approved by CWW.
- 2.5 In developed and undeveloped areas, vegetation shall be re-established in all disturbed areas by seeding and mulching, or by sodding as directed by the ENGINEER.
- 2.5.1 Prior to seeding or sodding, noxious weeds and Johnson grass shall be grubbed, and raked. The collected material shall be removed from the site. The surface shall be cleared of stumps, loose surface rocks larger than ¾", roots, trash and debris that might hinder grass mowing.
- 2.5.2 The CONTRACTOR shall provide high quality sandy loam topsoil with no rocks or noxious weeds. The topsoil shall be approved by the ENGINEER prior to application.
- 2.5.3 Thickness of topsoil shall be three (3") to six (6") inches.
- 2.5.4 The CONTRACTOR shall prepare a loose surface seed bed in the area of replacement, to ensure a minimum depth of two inches (2").
- 2.5.5 Areas to be seeded and mulched shall be accomplished by seeding with permanent lawn grasses. Seed shall comply with the current rules and regulations of the Arkansas State Plant Board and the germination test shall be valid on the date the seed is used. It shall have a minimum of 98% pure seed and 85% germination by weight, and shall contain no more than 1% weed seeds. Do not use FESCUE or WHEAT seed. Seed shall be placed on a loose surface seedbed. Fertilizer (equivalent analysis 10-20-10) shall be applied to these areas at the rate of 250 lbs. per acre. Grass mixture shall be as approved by CWW. SEEDING AND MULCH SHALL BE OF THE HYDRO-SEEDING METHOD. Mulch shall be finely chopped. Mulch cover shall consist of; straw from threshed rice, oats, wheat, barley, or rye; wood excelsior; hay obtained from various legumes or grasses, such as lespedeza, clover, vetch, soybeans, bermuda, carpet sedge, bahia, fescue, or other legumes or grasses; or a combination thereof. Mulch shall be dry and reasonably free from Johnson grass or other noxious weeds, and shall not be excessively brittle or in an advanced state of decomposition. Mulch shall be applied at the rate of 3000 lbs. per acre. Tackifiers used in mulch anchoring shall be of such quality that the mulch cover will be bound together to form a cover mat that will stay intact.
- 2.5.6 The Contractor shall replace seed and mulch that has eroded or failed to germinate until the ground has "established" vegetation.
- 2.5.7 Property with existing sod shall be re-sodded. New sod shall be of the same species as existing turf.
- 2.6 Ornamental trees and shrubs having a diameter of two inches (2") or less (measured 6-inches from the ground) shall be removed and replaced. The CONTRACTOR shall hand dig around the tree or shrub to be replaced. The entire root system shall be removed (minimum 2-foot diameter root ball), wrapped in burlap, moistened and protected.

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- 2.6.1 The CONTRACTOR shall replant/replace any tree or shrub removed within 48-hours of removal.
- 2.6.2 A hole shall be excavated sufficient to accommodate the entire root system. The hole shall be filled with rich topsoil. This material shall be compacted below and around the root system.
- 2.6.3 Trees or shrubs shall be guaranteed to live for one year after being replanted or replaced. Dead or dying trees or shrubs shall be replaced within thirty (30) days of written notification at the CONTRACTOR'S expense.
- 2.6.4 The CONTRACTOR shall thoroughly water all planted and sodded areas and maintain them by application of additional water and additional fertilizer until an average of 80% of the ground has "established" vegetation.
- 2.6.5 Restoration of permanent summer lawn grass will be considered complete when the grass has a well-developed root system showing vigorous runners in all directions and blending with the undisturbed grass adjacent to the area being re-established.
- 2.6.6 CWW will furnish metered water necessary for the establishment of vegetation.
- 2.6.7 Backflow Preventers shall be furnished and installed by CWW between the water source and the hose to be used. Removal or relocation of the Backflow Preventer(s) is prohibited.
- 2.6.8 All lawns and maintained grass areas, pastures and meadows, and wooded areas disturbed by construction shall be restored to a condition equal to or better than that existing prior to construction. In general, all surfaces shall be graded smooth and shall be free of rock larger than ¾", clumps, sticks, trash and debris. Wooded areas cleared and grubbed of trees shall be only seeded and mulch unless otherwise directed by the ENGINEER.
- 2.6.9 Sodding and seeding work shall commence within one month of the installation of pipelines segment. Transplantation of vegetation shall be immediate after installation of pipe.
- 2.6.10 Failure to begin vegetation restoration in a timely manner or failure to complete vegetation restoration to the satisfaction of the property owner and ENGINEER shall result in CWW securing a licensed landscaping service to perform the work. The cost of the landscaping work will be deducted from monies due the CONTRACTOR.
- 2.6.11 The CONTRACTOR shall use suitable equipment for fine grading and landscape work. Heavy equipment normally associated with utility work is not deemed satisfactory. Equipment of the size and nature normally associated with landscape work shall be used.
- 2.6.12 All other work associated with the restoration of vegetation shall be subsidiary to the various pay items and will not be paid for separately.

### **3. GENERAL WATER & WASTEWATER PIPELINE REQUIREMENTS**

- 3.1 These general and detailed specifications form a part of the Contract Documents and shall govern the handling and installation of the pipe, valves, hydrants and appurtenances.
- 3.2 The approximate quantities are shown in the Plans and on the Bid Form.
- 3.3 Contractor shall comply with all requirements of the Occupational Safety and Health Administration's (OSHA) Excavation and Trenching Safety standard, Title 29 of the Code of Federal Regulations (CFR), Part 1926.650.
- 3.4 The CONTRACTOR shall secure and provide, at his expense, any special permits, including construction access, utility easements and rights-of-way; blasting permits and bonds; material storage permits; waste material disposal sites; and all other permits and licenses necessary for construction of the facilities within the scope of this project.
- 3.5 CWW shall secure highway permits and such other permits and licenses within its jurisdiction and pertaining to occupancy and use of the facilities to be constructed.
- 3.6 The CONTRACTOR shall excavate and install tapping sleeves, tapping valves and tap machine as required. CWW shall be notified in advance so as to observe the pipeline tap and witness the tap coupon retrieval.
- 3.7 **The CONTRACTOR shall, furnish all materials, labor, tools and equipment necessary to complete the project, including;**
  - a). All pipe, valves, hydrants, necessary fittings and appurtenances for mains and services.

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- b). All select backfill materials as needed, concrete for blocking
  - c). All sewer pipe materials and appurtenances, adapter gaskets and concrete for manholes, manhole rings, lids. All material and equipment for sewer lift stations.
  - d). All materials necessary for restoration of streets, lawns, etc.
  - e). All materials for trench shoring, traffic control signs and barricades, pipes and materials for repair or replacement of sewers, drains, and other utility structures and all other materials, necessary for satisfactory completion of the project as specified.
- 3.8 The CONTRACTOR shall clear the pipeline right-of-way; remove pavement as may be necessary; excavate the trench and pits to the required dimensions; construct and maintain all approaches required for traffic control; sheet, brace, and support adjoining ground or structures where necessary; handle all drainage or ground water; guard the site; load, unload, haul, distribute, lay and test the pipes, fittings, valves, hydrants, and appurtenances; rearrange branch connections to main sewers, conduits, ducts, culverts, or pipes where necessary; replace all damaged drains, sewers, culverts, or other structures; backfill the trench and pits; restore the roadway surface unless otherwise stipulated; remove surplus excavated material; clean the site of the work; and maintain the street or other surface over the trench until final acceptance of the completed project.
- 3.9 The CONTRACTOR shall be responsible for cost of inspectors and inspection fees.
- 3.10 Work shall not be performed on Saturdays, Sundays or legal holidays, except by written permission of an official of CWW.
- 3.11 All materials furnished by CONTRACTOR are subject to approval by the Engineer as to quality and suitability for the construction to be performed.
- 3.12 All work is to be performed with diligence in a workmanlike manner according to best practices so as to produce a completed job of a quality commensurate with prevailing standards of the water and wastewater industry and CWW particularly.
- 3.13 Water and Wastewater Mains are to be installed in accordance with AWWA Standards C600 for Ductile Iron Pipe, AWWA C605 for PVC pipe as applicable unless specifically provided otherwise by these specifications or by the Engineer in writing.
- 3.14 CONTRACTOR is an "Independent CONTRACTOR" and has full power and authority to select the means, methods, and manner of performing the work, subject to strict compliance with requirements of local authorities, being responsible to CWW for all materials delivered and for the results herein contracted for, and, upon completion of the contract, the work shall be delivered complete and undamaged in accordance with the specifications and plans.
- 3.15 CONTRACTOR shall not be permitted to sublet, sell or assign this contract or sublet any of the work to be performed hereunder without the written consent of CWW and any such assignment or subletting of any such work without said consent shall be null and void and without force and effect. CWW shall have the right to assign in whole or in part its rights hereunder.
- 3.16 CONTRACTOR shall prosecute the construction of said work with due diligence and at such a rate and in such manner as in the opinion of the Engineer is necessary for completion within a reasonable time. CONTRACTOR shall not open up work to the prejudice of work already started and shall arrange its work and dispose of materials so as to insure the least possible interference and inconvenience to the land owners on or beside whose property the pipelines are being constructed or to the public where the pipelines lie in or near a public thoroughfare. CONTRACTOR shall employ such number of construction crews as are reasonably necessary to construct said pipelines within the allotted time, provided CWW may require the employment of an additional crew or crews, if in its judgment it is necessary, in order to complete said pipelines within the time required.
- 3.17 CONTRACTOR shall furnish the Engineer with every reasonable facility for ascertaining whether the work performed is in accordance with the requirements and intent of the contract agreement, plans, and specifications. If the Engineer requests it, CONTRACTOR at any time before acceptance of the work by CWW, shall remove or uncover such portions of the finished work as may be directed. After examination, CONTRACTOR shall restore said portions of the work to the standards required by the Specifications. Should the work thus exposed or examined prove to be in accordance with the Specifications, the uncovering or removing, and the replacing of the covering or making good of the parts removed shall be paid for by CWW, but should the work so exposed or examined prove to be not in accordance with the Specifications, the

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uncovering or removing and the replacing of the covering or the making good of the parts removed, shall be at Contractor's expense. Should any work be performed without giving notice of plan of work and opportunity of inspection by the Engineer or his representative, the Engineer may require CONTRACTOR to uncover such work at his own expense for examination by the Engineer. Cost of uncovering such work shall be borne by the CONTRACTOR, whether or not the work is found acceptable. The work shall also be subject to inspection by other appropriate governmental inspectors at all times.

- 3.18 CONTRACTOR at all times shall have a Superintendent as its agent of the work. Such Superintendent shall have full authority to execute the orders, or directions, of the Engineer without delay; shall apply promptly such materials, tools, equipment and labor as may be required; same shall be furnished irrespective of the amount of work sublet; and shall not be removed from the work, or replaced, without written permission by the Engineer.
- 3.19 Except in instances where common labor may be utilized, CONTRACTOR shall employ only competent, experienced, and skilled workman to do the work, and all work shall be performed and completed in good, careful, and workmanlike manner and to the satisfaction of CWW. CWW shall have the right at all times to require the removal of any superintendent, foreman or workman for performing inferior work.
- 3.20 The methods, equipment and appliances shall produce a satisfactory quality of work and shall be adequate to maintain the schedule of progress as submitted under provisions herein above. CONTRACTOR shall maintain all equipment in good repair to insure efficient performance of the work required. The Engineer shall have the right to require the removal and/or replacement of any equipment which he deems incapable of satisfactory performance.
- 3.21 CONTRACTOR shall be responsible for any damage to CWW' equipment and facilities resulting from Contractor's negligence.
- 3.22 The CONTRACTOR shall be responsible for any damage to any existing facilities presently in service.
- 3.23 The CONTRACTOR shall be responsible for determining exact locations of all other utilities.

#### 4. MATERIALS

- 4.1 All water and wastewater pipe and service line material shall conform to the following specifications.
- 4.2 PVC pipe shall be manufactured by a member of the Uni-Bell Plastic Pipe Association.
- 4.3 All pipe and pipe fittings furnished and installed on this project shall be inspected and tested by the manufacturer. The manufacturer shall furnish to the Engineer, prior to delivery, certificates stating that all pipe is manufactured in compliance with these Specifications. The certificate shall also fully describe the pipe proposed to be furnished.
  - i. If evidence appears that all provisions of the applicable AWWA and ASTM Standards have not been complied with after the pipe has been delivered, CWW will require such field testing and sampling as necessary for certified statements of compliance to the provisions of said standards to be furnished by an approved independent laboratory. The cost for the testing and sampling or job delay will be the responsibility of the Contractor. The independent laboratory shall be one which may be chosen by the pipe manufacturer and approved by CWW.
- 4.4 All wastewater pipeline and wastewater service line material shall conform to the following specifications:
  - a). **Gravity Sewer Pipe and Fittings shall conform to one of the following ASTM standards:**
    - **ASTM D3034, PVC Sewer Pipe (4" through 15" diameter), SDR-35, Green Pipe**, with a minimum Pipe Stiffness of 46 psi as determined by ASTM D2412.
    - **ASTM F679, PVC Large Diameter Sewer Pipe (18" through 48" diameter)** with a minimum Pipe Stiffness of 46 psi as determined by ASTM D2412. The fittings and pipe shall be of a gasket push on joint type meeting ASTM D3212.
    - **ASTM F794, PVC Profile Gravity Sewer Pipe and Fittings** with a minimum Pipe Stiffness of 46 psi as determined by ASTM D2412 (only for diameters larger than 12").
    - **ASTM F949, PVC Corrugated Sewer Pipe** with smooth interior and Fittings with a minimum Pipe Stiffness of 46 psi as determined by ASTM D2412 (only for diameters larger than 12").

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- **Sewer Pipe** shall have integral bell joints with elastomeric gaskets that conform to ASTM 3212 and ASTM F477.
  - b). Sewer **service** pipe material shall conform to **ASTM D 2665, Schedule 40 DWV**.
  - c). Sewer service saddles: Acceptable Manufacturer, ROMAC Industries Style “CB” or equal
  - d). Sewer Force Main Pipe:
    - **ASTM D2241, SDR-21** pressure rating of 200 psi.
    - or **ASTM D1785 Schedule 40** for size 1-4” diameter.
    - or Polyethylene (PE) Pressure Pipe and Fittings 4 inch through 63 inch for Water Transmission and Distribution: AWWA C906, DR11, Pressure Class 160 PE3408, Ductile Iron OD (DIOD).
  - e). Air/Vacuum release valves: Acceptable Manufacturer is A.R.I. model D-025 or equal
- 4.5 **All water pipeline and service line material shall conform to the following specifications:**
- a). **PVC Pressure Pipe and Fabricated Fittings 4” through 12”:** **AWWA C900, DR18 or DR14.** **ASTM D2241 Pressure-Rated Pipe (Commonly referred to as SDR21 Class 200) is not permitted.** PVC Water Pipe shall have integral bell joints with elastomeric gaskets that conform to ASTM 3212 and ASTM F477.
  - b). **Ductile Iron water pipe** (flanged, mechanical joint and push-on joint) shall be bituminous coated (Asphaltic outside coating), cement lined, Class 50 conforming to **AWWA C151 & C104**.
  - c). Ductile Iron and Gray Iron water pipe fittings: AWWA C110/A21.10, Pressure rating 250 psi.
  - d). Rubber-Gasket Joints For Ductile-Iron Pressure Pipe and Fittings: AWWA C111/A21.11
  - e). Flanged Ductile-Iron Pipe with Ductile-Iron or Gray-Iron Threaded Flanges: AWWA C115/A21.11, thickness class 53, 250 psi working pressure
  - f). Ductile-Iron Compact Fittings (Mechanical Joint): AWWA C153/A21.53
  - g). Double Check Valve Backflow-Prevention Assembly: AWWA C510
  - h). Air/Vacuum release valves: Acceptable Manufacturer is A.R.I. model D-025 or equal
  - i). Gate Valves: AWWA C509, Resilient Seated, open left (counterclockwise) with a 2” nut, epoxy coated. Acceptable Manufacturers are: Mueller, Clow, American Flow Control, American AVK.
  - j). Fire Hydrants: AWWA C502, open left (counterclockwise) with a 1 ½” pentagon nut, 1” tall, three way outlets 2-2 ½” hose nozzles and 1-4 ¼” pumper nozzle, 5 ¼” main valve. Fire Hydrants shall be painted by the Manufacturer - **Safety Yellow**. Cabot WaterWorks will not approve field painting of Fire Hydrants.  
  
Acceptable Manufacturers are: Mueller Super Centurion, Clow Medallion, American Flow Control (“AFC-PACER”), American AVK.
  - k). Water Service Line Valves and Fittings AWWA C800
  - l). Reduced Pressure Principle Backflow-Prevention Assembly (RPZ): AWWA C511
  - m). Polyethylene (PE) Pressure Pipe and Tubing, ½ In. Through 3 In, for Water Service: AWWA C901, DR9, Pressure Class 160. The minimum size for water service lines for long services defined as service on the opposite side of street as water main line shall be 1.5” diameter, except on individual metered apartments. Short services defined as services on the same side of street as water main shall be a minimum of 1 inch diameter.
  - n). Polyethylene (PE) Pressure Pipe and Fittings 4 inch through 63 inch for Water Transmission and Distribution: AWWA C906, DR11, Pressure Class 160 PE3408, Ductile Iron OD (DIOD).
  - o). PVC Water Service Pipe 1” through 2” diameter - ASTM D 1785, Schedule 40 PVC - PVC1120 or 1220 or 2120.
  - p). **Tapping Sleeves:** 18-8 Type 304 Stainless Steel, Manufacturers; Ford, Mueller, Smith Blair
  - q). **Service Saddles:** Brass or 18-8 Type 304 Stainless Steel, Manufacturers; Ford, Mueller, Smith Blair

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r). **Meter Setters:**

<b>Meter Setters</b>	<b>FORD Meter Box Part #</b>
3/4" DP x 3/4" DP Coppersetter	VB73 - 8W - 41 - 33
1" PJ x 3/4" DP Coppersetter	VB74 - 10W - 41 - 43
1" PJ x 1" DP Coppersetter	VB74 - 10W - 41 - 44
1 1/2" FIP x 1 1/2" FIP Coppersetter	VBB 76 - 12B - 11 - 66
2" FIP x 2" FIP Coppersetter	VBB 77 - 12B - 11 - 77
1" PJ x 3/4" MIP x 3/4" MIP U-Branch	U48 - 43 - 7-1/2"

s). **Meter Boxes** Manufacturer: East Jordan Ironworks:

Single, 5/8 x 3/4" and 3/4" Meters:

#11P Box with Cast Iron Drop in Lid

Or

BC 1015 Box with Cast Iron Lid or Cast Iron Lid with Reader Lid

Or

BCF 1118 with Cast Iron Lid or Cast Iron Lid with Reader Lid

Dual or Double, 5/8 x 3/4" or 3/4" Meters:

BC 1416 Box with Cast Iron Lid or with Cast Iron Lid with Reader

Or

BCF 1416 Box with Cast Iron Lid or with Cast Iron Lid with Reader

1" Meter Sets:

#15P Box with Cast Iron Drop in Lid

Or

BCF 1320-14 Box with Cast Iron Lid

1 1/2" and 2" Meter sets:

MSBCF 1527 Meter Box with 2 - 65T Dual Cast Iron Lids or Cast Iron Lid or Cast Iron Lid with Reader

Or

MS 1730 with reader lid.

- 4.6 All materials in contact with drinking water shall be certified as being in compliance with **ANSI/NSF 60 and 61** by an independent agency meeting the criteria specified by the Arkansas Department of Health
- 4.7 All materials are subject to inspection and approval at the plant of the manufacturer at the discretion of CWW
- 4.8 During the process of loading or unloading, all materials shall be inspected by the CONTRACTOR for damage and any damaged material set aside.
- 4.9 Inspection of material at the manufacturer's plant, at the point of delivery, on the job site, or in place shall not relieve the CONTRACTOR of his responsibility and the material may be subject to rejection until final acceptance of the completed project by CWW.
- 4.10 HANDLING PIPELINE MATERIALS
- The CONTRACTOR shall handle the material with the utmost care and in a manner to prevent damage to the materials, material coating and lining during loading, hauling, unloading, and installation operations. Pipe and fittings shall not be dragged along the ground or dropped and shall be stored so as to protect all materials from damage. Material damaged shall be replaced or repaired at the CONTRACTOR'S expense.
  - Hooks shall not be in contact with the pipe interior.
  - The interior of the pipeline materials shall be kept free from dirt and foreign matter at all times.

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- iv. Pipeline materials, especially valves, hydrants and fittings shall be drained and stored in a manner to protect them from damage by freezing.

### 5. LOCATION, ALIGNMENT and GRADE

- 5.1 The pipe, fittings, valves and hydrants shall be located and maintained to conform closely to the location, lines and grades specified hereunder or as shown on the Plans, whichever is more stringent.
  - i. Water and wastewater main alignment shall be:
    - a). Parallel to and within the outer 4' of street or highway right-of-way or in the center of easement boundaries.
    - b). Water and wastewater mains shall typically be installed on opposite sides of streets.
    - c). Water and wastewater mains shall have a minimum clearance of 3' (feet) from all other utilities.
    - d). CWW shall approve alignment and grade of water and wastewater mains prior to construction.
  - ii. **Water Mains shall be installed with 3' minimum cover and 4' maximum cover below final grade.**
  - iii. Main line resilient seated gate valves of the same diameter as the pipe shall be installed on both sides of connections to lateral water mains and every 1200' along water mains, typically at locations where fire hydrants are installed. Gate valves shall be installed on all lateral water mains within 10 feet of connection to mains.
  - iv. Fire Hydrants shall be installed at a maximum spacing of every 600' along water mains and at intersections to side streets.
  - v. **Sanitary hazards:** Water mains and sanitary sewers shall be constructed as far apart as practicable, and shall be separated by undisturbed and compacted earth. A minimum horizontal distance of ten feet should be maintained between water lines and sewer lines or other sources of contamination. Water lines and sewers shall not be laid in the same trench except on the written approval of the Arkansas Department of Health. Water mains necessarily in close proximity to sewers must be placed so that the bottom of the water line will be at least 18 inches above the top of the sewer line at its highest point. If this distance must unavoidably be reduced, the water line or the sewer line must be encased in watertight pipe with sealed watertight ends extending at least ten feet either side of the crossing. Any joint in the encasement pipe is to be mechanically restrained. The encasement pipe shall be vented to the surface if carrying water or sewer under pressure. Where a water line must unavoidably pass beneath the sewer line, at least 18 inches of separation must be maintained between the outside of the two pipes in addition to the preceding encasement requirement. Exceptions to this must be approved in writing by the Arkansas Department of Health.
  - vi. Valves and hydrants shall be set with operating stem and nut plumb. There shall be no sharp and sudden breaks, requiring extra fittings and no joint shall be located underneath a sub-structure without the consent of the ENGINEER.
  - vii. Horizontal and vertical (where required) control points will be established along or adjacent to the construction area. It shall be the responsibility of the CONTRACTOR to make necessary measurements from these control points in order to maintain the proper alignment and grade of the structures. The CONTRACTOR shall preserve all stakes and markers established by the ENGINEER. Failure of the CONTRACTOR to preserve such stakes and markers will result in the cost for re-establishment being deducted from amounts due CONTRACTOR.
  - viii. The Contractor shall be responsible for resetting all property pins disturbed by construction activities.

### 6. TRAFFIC CONTROL & PUBLIC TRAVEL

- 6.1 The CONTRACTOR shall plan and execute the work so to minimize interference with flow of traffic and passage of pedestrians.
- 6.2 **Closing streets with one access route is not permitted.** One traffic lane shall remain open at any given time. Excavated areas within the traffic lanes of highways, streets or roads and pedestrian walkways shall be backfilled as soon as possible and the area reopened to traffic.
- 6.3 The CONTRACTOR shall be responsible for making provisions for the safe and free passage of persons and vehicles by, over, or around the work while it is in progress. Such provisions or traffic control plans shall be

## **Cabot WaterWorks Standard Construction Specifications**

satisfactory with the ENGINEER, State, County and Local authority having jurisdiction within the area of work.

- 6.4 The CONTRACTOR shall notify applicable State, County or local authority before closing or obstructing any public highway, street or road.
- 6.5 When required by the State, County or local authority having jurisdiction therefore, that traffic be maintained over any construction work in a public highway, street or road, and such traffic cannot be maintained on the alignment of the original roadbed or pavement, the CONTRACTOR shall, at his own expense, construct and maintain a detour around the work. Such detours shall be satisfactory with the ENGINEER and State, County or local authority.
- 6.6 The CONTRACTOR shall provide and maintain necessary barricades, signs, torches, lights and markers around the work in order to protect persons from injury and avoid property damage. The CONTRACTOR shall also provide qualified flagmen to direct traffic while working upon a highway, street or road over which traffic must pass.
- 6.7 The CONTRACTOR shall make the same provisions as described above for the passage of vehicular and pedestrian traffic between private property and public highways, streets and roads or other provisions that are satisfactory to the ENGINEER and the property owners involved.

### **7. LOCATION & PROTECTION OF ABOVE GROUND & BURIED FACILITIES**

- 7.1 The approximate location of buried facilities shown on Plans is for the guidance of the CONTRACTOR. CWW does not guarantee the accuracy or correctness of the locations of such structures as shown. Furthermore, there may be structures that are not shown.
- 7.2 It shall be the responsibility of the CONTRACTOR to determine the actual location of all buried facilities.
- 7.3 The CONTRACTOR shall make necessary exploratory excavations to determine the location of underground facilities such as pipes, drains, conduits, cables and other structures.
- 7.4 The Contractor shall be responsible for contacting the owners of such facilities before excavating in the vicinity of these facilities and shall be guided by their instructions.
- 7.5 The CONTRACTOR shall provide adequate protection and support for all above ground and buried facilities or other facilities encountered during the progress of the work. Whenever such structures or facilities are in the same location as the proposed pipeline or appurtenances thereto, the CONTRACTOR shall relocate or reconstruct or cause to be relocated or reconstructed, the structure or facility to the satisfaction of the ENGINEER and utility facility owner. Whenever requested by the ENGINEER or utility owner, the CONTRACTOR shall provide drawings and other plans for supporting or otherwise safeguarding surface and buried facilities or other facilities which, in the opinion of the ENGINEER, or utility or facility owner, may be damaged as a result of the CONTRACTOR'S work.
- 7.6 The CONTRACTOR shall not stop or impede the flow in any pipe, sewer, surface or subsurface drain without making provisions for diverting the flow to the satisfaction of the ENGINEER.
- 7.7 If any utility facility or structure is damaged during the progress of the work, the CONTRACTOR shall immediately notify the appropriate owner. Repairs shall not be made by the CONTRACTOR without the prior approval of the utility facility or structure owner. The CONTRACTOR shall pay utility owners for the cost of repairing, relocating or replacing any facilities damaged by the CONTRACTOR. In addition, the CONTRACTOR shall provide all assistance available to the utility involved in making repairs under emergency conditions.
- 7.8 The CONTRACTOR shall not operate valves in the existing distribution system without approval of the ENGINEER.
- 7.9 All existing fire hydrants and bends shall be blocked or tied in such manner so as to prevent displacement before excavating behind these appurtenances.

### **8. CLASSIFICATION OF EXCAVATION Water & Wastewater**

- 8.1 All excavations shall be unclassified regardless of the nature of materials encountered.

## Cabot WaterWorks Standard Construction Specifications

### 9. PIPE TRENCH CONSTRUCTION Water & Wastewater

- 9.1 All water and wastewater pipes, (Ductile Iron and PVC) shall be bedded in accordance with Section 10, PIPE BEDDING & BACKFILL.
- 9.2 Unless bound herein or shown on the Plans, information related to soil conditions is not furnished by CWW. Any information furnished by CWW related to soil conditions is not guaranteed to represent actual conditions which the CONTRACTOR will encounter and is to be used at the CONTRACTOR'S risk; therefore, the CONTRACTOR shall take whatever action he feels necessary to satisfy himself as to the actual subsurface soil conditions.
- 9.3 The trench shall be excavated to the alignment and grade specified and only so far in advance of pipe installation as the ENGINEER shall permit. Trenches along public streets or roads shall be limited to 300 feet or less, or as permitted by the governmental authority having jurisdiction. This limit may be reduced by the ENGINEER in congested areas.
- i. The use of trench-digging machinery will be permitted except in places where operations of same will cause damage to trees, buildings, or other existing facilities above or below the ground; in which case hand methods shall be employed.
  - ii. CONTRACTOR shall remove the minimum amount of street, driveway, sidewalk, parking lot, or other pavement required to permit installation of the pipelines or appurtenances. All pavement surfaces shall be saw-cut in straight lines with suitable equipment before removal. Concrete surfaces shall be scored with a suitable concrete saw unless all material is removed between existing construction joints.
- 9.4 Blasting for excavation will be permitted only after the CONTRACTOR secures the approval of the ENGINEER and only when proper precautions are taken for the protection of persons and property. The hours of blasting will be fixed by the ENGINEER. Any damage caused by blasting shall be repaired by the CONTRACTOR at his expense. The method of transporting, handling, and storage of explosives and blasting procedure shall conform to Federal Regulations, local and state laws and municipal ordinances.
- 9.5 Excavation Safety
- i. Whenever necessary to prevent caving; excavations in sand, gravel, sandy soil, or other unstable material shall be adequately sloped, shored and/or braced in accordance with OSHA 29 CFR 1926.650 SUBPART-P.
  - ii. **29CFR 1926.652** - Each employee in an excavation shall be protected from cave-ins by an adequate protective system designed in accordance with 29 CFR 1926.650 except when:
    - Excavations are made entirely in stable rock; or
    - Excavations are less than 5 feet (1.52 m) in depth and examination of the ground by a competent person provides no indication of a potential cave-in.
- 9.6 Where shoring and bracing is used, the trench width may be increased accordingly. Trench shoring shall remain in place until the pipe has been laid and jointed. Where slides or cave-ins occur, the CONTRACTOR shall, at his expense, provide proper bedding and support for the pipe.
- 9.7 The trench width may vary and depend upon the depth and the nature of the excavated material encountered. The trench shall be of ample width to permit the pipe to be laid and jointed properly and the backfill to be placed and compacted properly.
- i. The minimum width of trench shall be
    - a). **12" plus the nominal pipe diameter for pipe up to 24-inches in diameter;**
    - b). 18" plus the nominal pipe diameter for pipe over 24-inches in diameter.
- 9.8 The trench shall be excavated to a depth that accommodates **3' (feet) of cover** measured from top of pipe to finished grade plus undercut as required where the following conditions exist:
- a). When ledge rock, boulders, large stones, and other rock formations are encountered, all rock shall be removed to provide a clearance of at least 6" (inches) below and sides of pipe, valves, and fittings.
  - b). 9" inches when Type 5 Bedding is required.
  - c). 9" inches for pipelines and appurtenances larger than 24-inches in diameter.

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- d). Where the bottom of the trench at sub-grade is found to be unstable or to include ashes, cinders, refuse, vegetable or other organic material, or large pieces of inorganic material, that, in the judgment of the ENGINEER, should be removed, the CONTRACTOR shall remove all such material to the extent required by the ENGINEER.

9.9 All excavated material shall be stockpiled in a manner that will not endanger the work and that will not obstruct streets, sidewalks and driveways. Excavated material that is acceptable for pipe bedding shall be separated from the general excavation material. Excess or unsuitable material and debris shall be removed promptly. Gutters and ditches shall be kept clear or other satisfactory provisions shall be made for drainage.

**10. PIPE BEDDING & BACKFILL Water & Wastewater**

10.1 CONTRACTOR shall bed all pipe in accordance with Type 3, Type 4 or Type 5 bedding details below.

- a). Type 5 Bedding shall be required at any street, road, or ally crossing.
- b). Type 4 Bedding shall be required where excavated trench material contains rock larger than ¾”.
- c). Type 3 Bedding maybe used where excavated native trench material contains no rocks larger than ¾”.
- d). State Highway crossings shall be performed by tunneling and/or boring in accordance with Section 24.

10.2 **All pipe bedding shall be inspected by CWW’ representative before backfilling.**

10.3 All bedding shall provide a uniform and continuous support for the pipe barrel. A shallow depression shall be made in the bedding to accommodate bell ends.

10.4 If the pipe trench passes over previous excavations, the trench bottom shall be compacted to provide the same bearing capacity as adjoining undisturbed native soil.

10.5 All trenches shall be backfilled as soon as possible after installation of pipe and appurtenances. In areas with concrete blocking, it may be necessary to backfill only a portion of the trench in order to allow placement and curing of concrete. Fittings or appurtenances designated by the ENGINEER shall not be backfilled until the hydrostatic pressure and leakage tests have been completed.

10.6 Dona-fill material shall not be used for bedding or backfilling.

**10.7 TYPE 3 BEDDING**

- i. Where approved in advance by the ENGINEER, Type 3 bedding material may consist of the excavated trench material where the material is free from rocks or unyielding material larger than ¾” angular or 1.5” rounded and of proper moisture content for maximum consolidation. (Unyielding material is defined as any material that cannot be crushed or deformed with a bare hand).
- ii. If the excavated native material contains large rock or organic mater, then Type 4 bedding will be required as specified below.
- iii. Bedding material shall be deposited to provide a 6” layer under the pipe and carefully tamped and rammed around the pipe until the backfill has been brought to 6” above the top of the pipe.

**10.8 TYPE 4 BEDDING**

- i. Type 4 bedding shall be required where rocks or unyielding material larger than ¾” angular or 1.5” rounded are encountered or as directed by the Engineer.
- ii. **SELECT GRANULAR BEDDING MATERIAL** –shall conform to the following grading by weight:

SIEVE SIZE	PERCENT PASSING
¾”	100
#4	10 - 55
#40	3 - 30
#200	2 - 10

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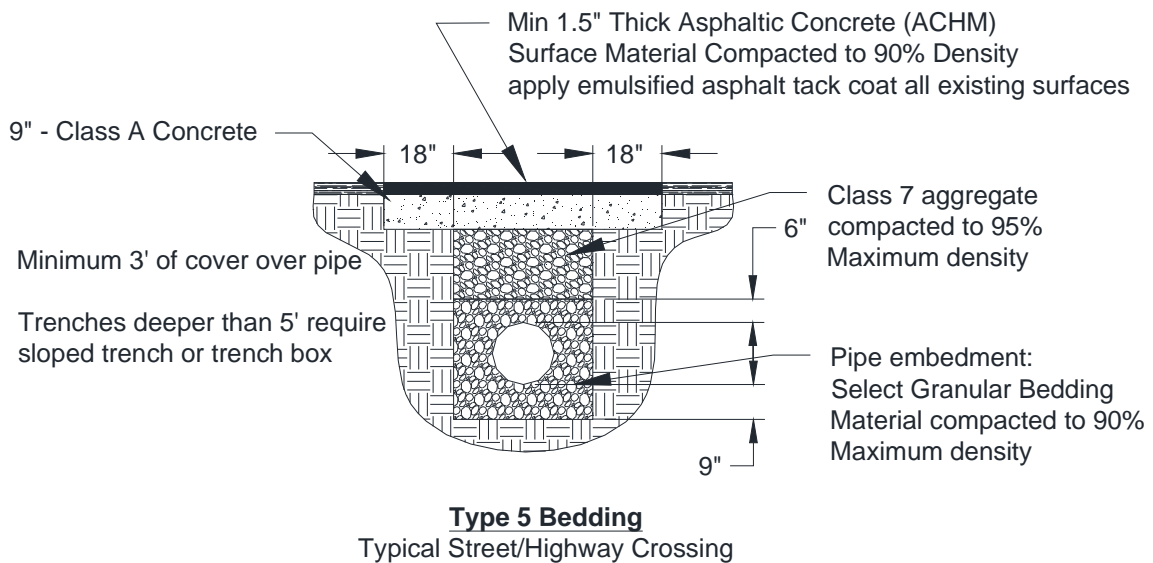
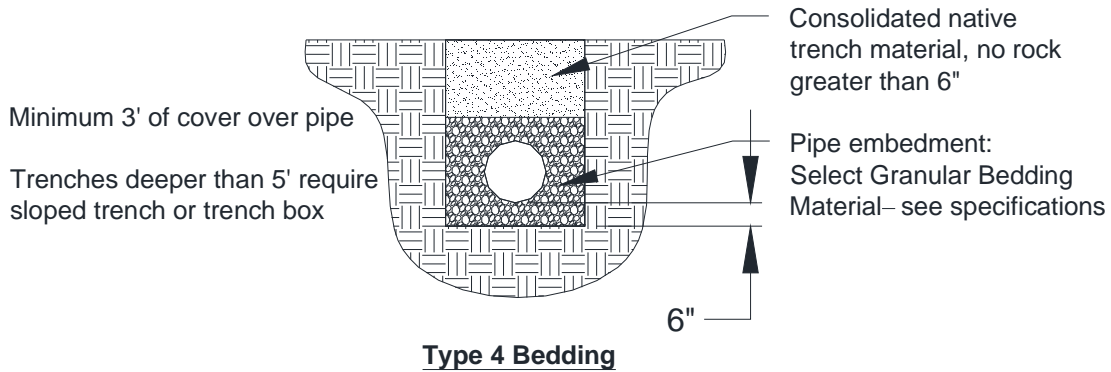
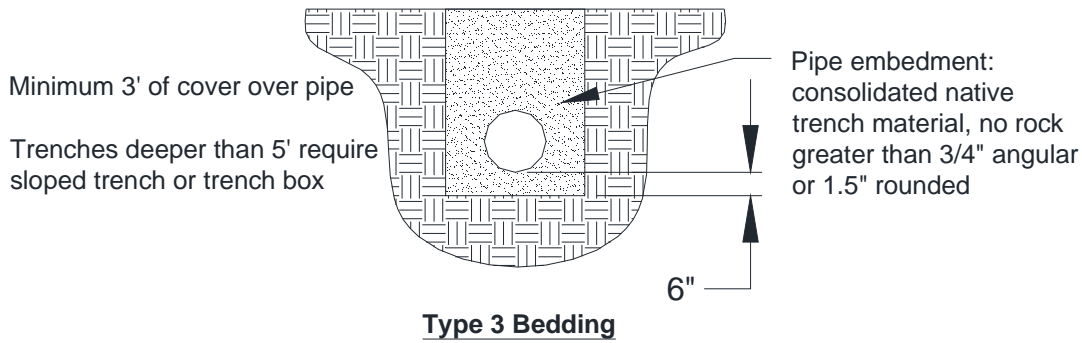
- iii. Select Granular Bedding Material shall be placed to provide a 6" bedding layer under the pipe and carefully tamped and rammed around the pipe until the aggregate has been brought to 6" above the top of the pipe.
- iv. The remaining trench may be backfilled with native material, except rocks larger than six (6) inches will not be permitted. Upon reaching the top of the trench with backfill, the trench shall be rolled by passing the wheel or track of heavy equipment along the trench line to attain consolidation.
- v. In areas where sod, trees, shrubs, flowers or other vegetation has been removed and is to be replanted, the top six (6) inches of backfill shall be of suitable top soil.

### **10.9 TYPE 5 BEDDING**

- i. Select Granular Bedding Material shall be deposited to provide a 9" layer under the pipe and carefully tamped and rammed around the pipe until the aggregate has been brought to 6" above the top of the pipe. Where the soil in the bottom of the trench is wet or soft enough to intrude into the voids of granular material, coarse sand shall be used for bedding.
- ii. After aggregate has been brought to 6" above the top of the pipe, compaction shall continue until 90% of maximum density as determined by the Modified Proctor Compaction Test has been achieved.
- iii. When trenches cross or pass through roads, streets, alleys, sidewalks, driveways, parking lots, or other areas specified, the backfill shall be continued using the Select Granular Bedding Material, and shall be compacted to 95% of maximum density as determined by the Modified Proctor Compaction Test to a point 9" below the bottom of the improved surface. As an alternate, the trench from 6" inches above the pipe to 9" below the bottom of the improved surface may be filled with a soil-cement mixture or a mixture of flyash-bottom ash, which has been approved by the ENGINEER in advance. The top 9" of backfill shall consist of pavement repair conforming to the detail for Type 5 bedding below.

10.10 The CONTRACTOR shall fill trenches with suitable material where settlement has occurred throughout the warranty period after completion of the project.

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### 11. PIPE INSTALLATION – GENERAL Water & Wastewater

- 11.1 The Contractor shall notify the Engineer or his representative 24 hours prior to trenching to schedule inspection of all pipe installed. Pipe bedding shall be inspected by CWW's representative prior to backfilling.
- 11.2 Proper implements, tools and equipment shall be provided and used by the CONTRACTOR for the safe and convenient prosecution of the work. Under no circumstances shall pipe or accessories be dropped or dumped into the trench.
- 11.3 All foreign matter or dirt shall be removed from the inside of the pipe and appurtenances before lowering into the trench and the pipe interior shall be kept clean during and after installation. A swab shall be kept in the pipelines as long as the pipe is being laid. Care shall be taken to prevent dirt from entering the joint space and

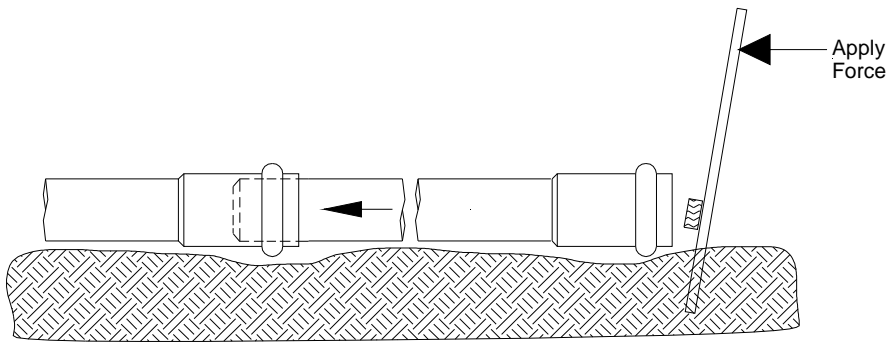
## Cabot WaterWorks Standard Construction Specifications

at times when pipe installation is not in progress, the open ends of the pipe shall be closed by installing a plug or cap of sufficient design to prevent trench water, foreign matter, and dirt from entering the pipeline.

- 11.4 Cutting of the pipe for inserting into valves, fittings or closure pieces shall be done in a neat and workmanlike manner without damage to the pipe or pipe lining. Torch cutting is not permitted. Pipe inserted into valves, fittings or closure pieces shall be **cut at an angle of 90° to the pipe centerline**. Cutting at other angles to provide greater deflections at the joints shall not be permitted. Field welding or welding except by the pipe manufacturer shall not be permitted.
- 11.5 Pipe shall be laid with bell ends facing the direction of installation; and for lines on an appreciable slope, bells shall, at the direction of the ENGINEER, face upgrate.
- 11.6 No pipe shall be laid in water, or when the trench condition or the weather is unsuitable for such work, except by permission of the ENGINEER. Contractor shall dewater open trenches as required prior for pipe installation.
- 11.7 **Tracer Wire:** A continuous 14-gauge copper wire (insulation color blue for water and green for sewer), shall be installed with all pipe including service lines. The wire shall be installed in the bottom of the pipe trench. The wire shall be looped up to the top of valve boxes, and around valves and other appurtenances in such manner that there is no interference with the operation of the valve or appurtenance.
  - i. Cast Iron Valve boxes or Cast Iron tracer wire access boxes shall be used to provide a connection point to tracer wires on long pipe runs every 1000'.
  - ii. Splices to tracer wire shall be joined by twisting together then installing a silicon filled twist on connector manufactured by DRYCONN "DBSR" or equal.
- 11.8 **Marking Tape:** All water and wastewater service lines and main line pipe shall have a detectable metallic marking tape, "Terra Tape" or equal installed 18" above the top of pipe or service line and labeled "Caution – Buried Water Line Below" for water lines and "Caution – Buried Sewer Line Below" for sewer lines. At the end of each service location, the Marking Tape shall be extended into meter boxes or up to finish grade for sewer services. The tape shall be in addition to the tracer wire.
- 11.9 **Waterline and Sewerline Sign Markers:** shall be installed every 1000 feet and at highway crossings, creek crossings, and changes in direction, railroad crossings or other areas of concern. Pipeline Signs shall conform to the detail below. Sign markers shall not be disturbed during construction.



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12.6 During the pipe laying operation, deflections at joints shall not exceed the amounts indicated in the following tables for the various types of joints and pipe.

12.7 PVC pressure pipe joint deflection may be achieved by following the procedure and limits, as defined:

- a). Keeping pipe length in straight alignment with the previously laid length, insert spigot end into the bell gasket until the reference mark is flush with the leading edge of the bell lip.
- b). Move to opposite end of the pipe and manually offset the pipe to the recommended offset. A block and bar may be used on heavier diameter pipe to provide continually controlled movement. Never use excavation equipment to obtain offset.
- c). Partially backfill installed pipe length to secure placement.
- d). If additional deflection is required, proceed from sequence 1, on next pipe length.
- e). **Caution:** Over insertion of spigot end and/or exceeding the recommended offset may create material stress at joint assembly. PVC fittings shall be used to achieve greater offsets.

### MECHANICAL JOINT PIPE (Ductile Iron Pipe)

<u>Size of Pipe</u>	<u>Degrees</u>	<u>Maximum Offset (inches)</u>	
		<u>18' Joint</u>	<u>20' Joint</u>
4" - 6"	7°	27"	30"
8" - 10" - 12"	5°	20"	22"
14" - 16" - 18" - 20"	3°	11"	12"

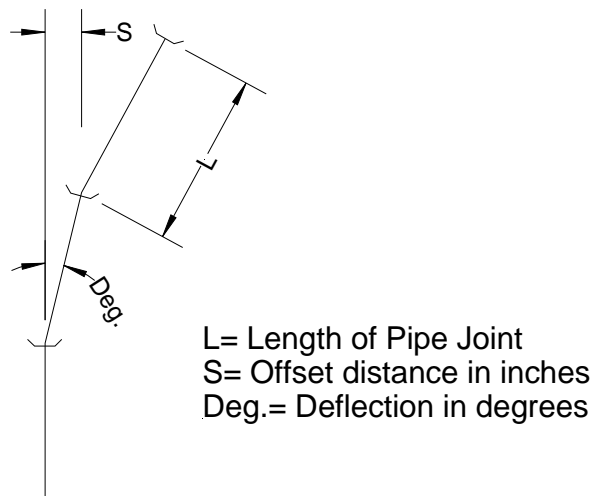
### PUSH-ON JOINT PIPE (Ductile Iron Pipe)

<u>Size of Pipe</u>	<u>Degrees</u>	<u>Maximum Offset (inches)</u>	
		<u>18' Joint</u>	<u>20' Joint</u>
4" - 6" - 8" - 12"	5°	19"	21"
14" - 16" - 20" - 24" - 36"	3°	11"	12"

### PUSH-ON PVC PIPE

<u>Size of Pipe</u>	<u>Degrees</u>	<u>Maximum Offset (inches)</u>	
		<u>20' Joint</u>	<u>Radius</u>
4" - 6" - 8" - 12"	3°	8"	573'
14" - 16" - 20" - 24" - 36"	1.5°	6"	764'

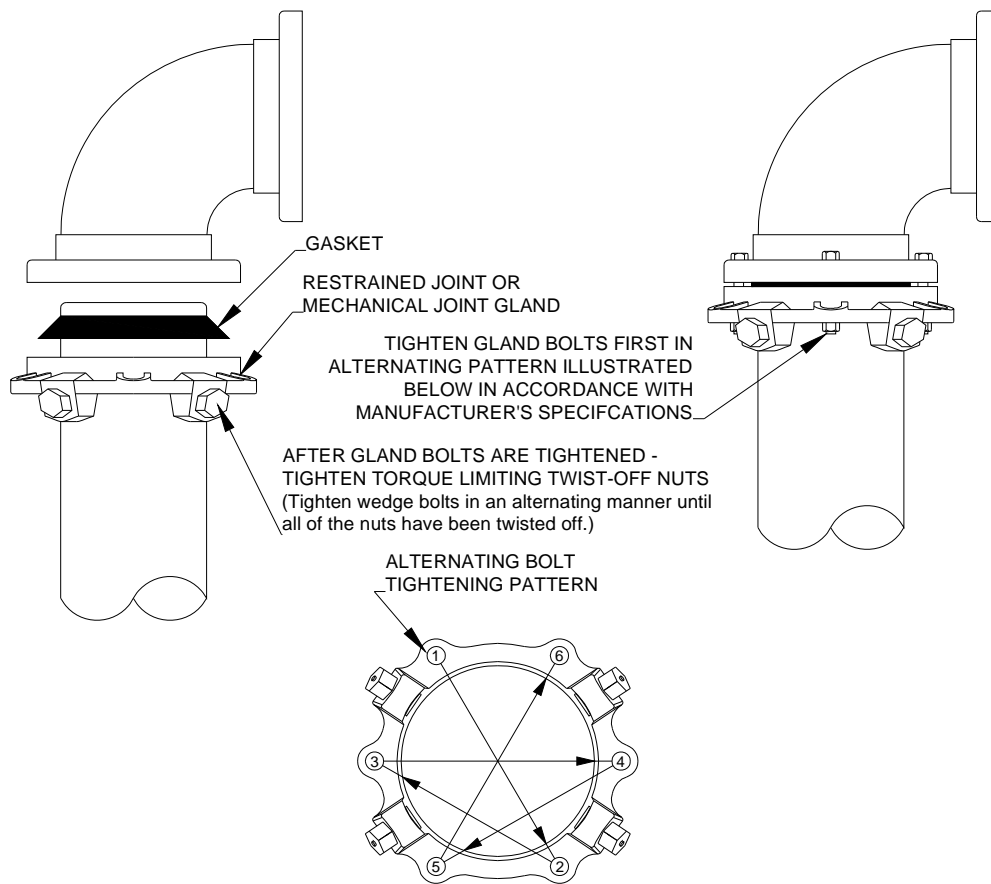
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### 13. MECHANICAL JOINT & RESTRAINED JOINT PIPE & FITTINGS

- 13.1 Beveled pipe ends shall be cut off for joining mechanical joint & restrained joint fittings
- 13.2 Prior to jointing pipe and/or fittings, the plain ends of the pipe and the bells of the pipe or fittings shall be thoroughly cleaned, removing all foreign materials from the bells, especially the gasket seats.
- 13.3 The gland rings shall be placed on the plain end of the pipe or fittings, followed by the rubber gasket which has been thoroughly cleaned and lubricated.
- 13.4 The plain end of the pipe shall be placed in the bell, to which connection is to be made, and shouldered in back of the bell. The rubber gasket shall be advanced into the bell and seated in the gasket seat; next the gland ring shall be brought into contact with the gasket, and all bolts entered and nuts started. The pipe may then be given a maximum deflection as prescribed in the preceding table.
- 13.5 Joints shall be made tight by advancing the nuts with a wrench in an alternating pattern until a tight joint is made.
- 13.6 The CONTRACTOR shall provide a "torque wrench" suitable for measuring tension on bolts for at least such a time as the workmen making the joints have gotten the "feel" of the required tension. At no time should handles longer than those supplied by the wrench manufacturer be permitted. The fitting manufacturer's recommended torque range shall be used. If recommended bolt torques are not provided by the manufacturer use the following torque ranges may be used:
- |              |                  |
|--------------|------------------|
| 5/8" Bolts   | 45- 60 ft. lbs.  |
| 3/4" Bolts   | 75- 90 ft. lbs.  |
| 1" Bolts     | 85-100 ft. lbs.  |
| 1 1/4" Bolts | 105-120 ft. lbs. |
- 13.7 After the workmen have become accustomed to this torque, a socket wrench with a maximum 10-inch handle may be used.
- 13.8 The rubber gasket and joint bolts of mechanical joint restraint glands shall be installed in accordance with above. Set bolts shall be tightened in an alternating pattern evenly to the manufacturers recommended torque.
- 13.9 The retainer gland and all bolts shall be encased in polyethylene material in accordance with Section 23.

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### Mechanical Joint & Restrained Joint Connections

#### 14. JOINTING FLANGED PIPE & FITTINGS

- 14.1 The faces of all flanges shall be thoroughly cleaned and all burrs or imperfections removed and brushed with a steel brush.
- 14.2 Gaskets between flanges shall be rubber or asbestos of 1/16-inch minimum thickness.
- 14.3 Care shall be taken to prevent strain of the flanges. All bolts and nuts shall be cleaned and lubricated prior to tightening. Bolts on opposite sides shall be tightened alternately to the torque listed in paragraph 13.6 herein.

#### 15. JOINTING POLYETHYLENE (PE) PIPE

- 15.1 PE pipe shall be joined by fusion, mechanical or flange adapter. The joining method used shall provide full thrust restraint for the pressure class of the pipe.
- 15.2 Cabot WaterWorks shall approve the joining material prior to construction.

#### 16. JOINTING ASBESTOS-CEMENT PIPE

- 16.1 Prior to jointing pipe, the entire machined surface and the interior of the couplings shall be thoroughly cleaned. Any burrs or deformations in any of the machined parts of the pipe shall be removed. The pipe which has been field cut shall be accurately machined using the appropriate machine tool.
- 16.2 Lubricant, furnished by the pipe manufacturer, shall be applied to the entire circumference of the pipe, from the end back to the shoulder stop. The lubricant shall be thoroughly applied using a hand, a cloth pad, a sponge or a glove.
- 16.3 The coupling shall be placed on the forward end of the pipe before lowering into the trench. The gasket recesses in the coupling must be completely free of dirt or other obstructions. The gaskets must be placed

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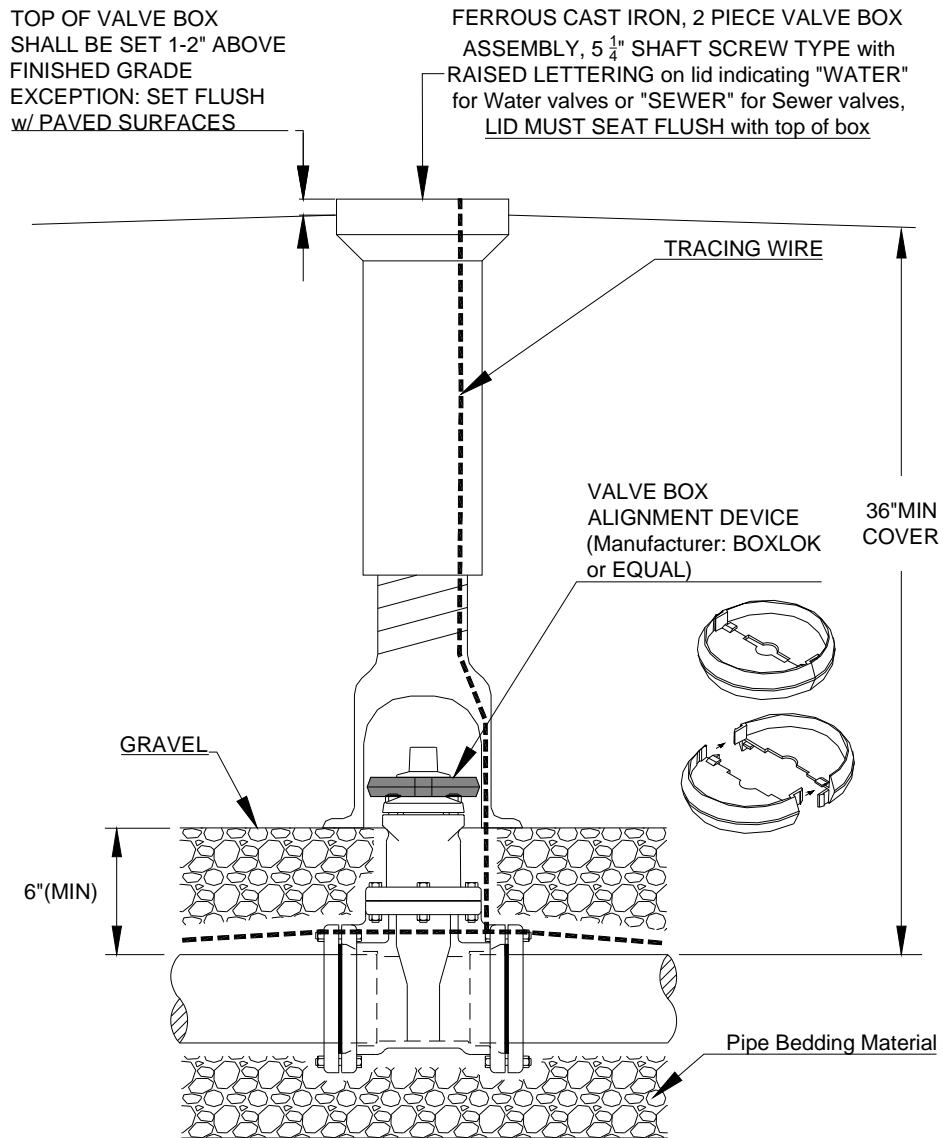
inside the groove, properly faced and completely seated. The couplings must be placed on the pipe by applying a constant force. Striking the coupling or pipe with a hammer or other heavy object will not be permitted.

- 16.4 After the pipe has been lowered into the trench, the machined end shall be inserted into the previously installed bell by the application of constant force on the piece of pipe being installed. Suitable levers or pullers shall be used. Care shall be exercised to assure that the pipe is not damaged by the application of pullers or levers. Suitable wood blocks shall be used between levers and the pipe.
- 16.5 After the joint has been assembled, a feeler gauge, supplied by the pipe manufacturer, shall be used to check the entire circumference of each side of the coupling to assure that the gasket is in the proper position. In case the gasket is not in the proper position, the pipe shall be non-jointed and the gasket checked. If the gasket is damaged, it shall be replaced. The pipe shall be lubricated, as specified above and re-jointed.

### 17. VALVE INSTALLATION

- 17.1 Valves shall be jointed in accordance with the methods of mechanical joint pipe as specified herein. Valve stems shall be plumb and there shall not be any obstructions which will prohibit the installation of valve boxes directly over the stem. Concrete anchor collars shall be provided around an adjoining length of pipe for all valves 16-inches in diameter or larger and for smaller valves when specified on the Plans. Mechanical joint retainer glands shall be installed on all valves with mechanical joint ends 12-inches in diameter or larger when ductile iron pipe is specified and for smaller valves when specified. All valves shall be firmly supported on well compacted soil.
- 17.2 Pipe shall be inserted as far as possible on each side of valves that are inserted in the line.
- 17.3 VALVE BOXES: Ferrous Cast Iron, 2 piece, screw type valve box assembly, 5 1/4" shaft with a cast iron lid shall be installed over all valves. The Cast Iron lid shall have "WATER" or "SEWER" cast into the lid with raised letters for water valves and sewer valves respectively. The Cast Iron lid shall seat flush with the top of the valve box. The valve box shall extend above grade at all times until completion of building construction and final grading. Valve boxes shall be installed over the operating nut of each valve and be of adequate length to reach the finished ground or paved surface. Boxes shall be firmly supported, plumb and centered over the valve operating nut using a valve box centering device. No part of the box shall rest on the valve, (See Valve installation detail below).

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### VALVE INSTALLATION DETAIL

Typical Valve installation Detail

## 18. TAPPING SLEEVE & TAPPING SADDLES

- 18.1 The Contractor shall notify CWW' representative 24 hours prior to connecting to CWW' distribution system. CWW representative shall be present to observe the pipeline tap and witness the tap coupon retrieval.
- 18.2 The CONTRACTOR shall excavate and install tapping sleeves, tapping valves and tap machine as required.
- 18.3 The pipe shall be free of dirt and other debris and shall have a smooth surface before attaching tapping sleeve or tapping saddle. All rough areas on the pipe barrel shall be smoothed.
- 18.4 Tapping saddles or sleeves shall be bolted securely to the pipe. The face of the outlet shall be plumb. Tapping sleeves shall be installed in accordance with manufacturer's procedures. Bolts for tapping saddles shall be alternately tightened "snug" and then alternately tightened to a torque of 100-foot pounds minimum.
- 18.5 The tapping valve shall be bolted securely to the tapping sleeve or tapping saddle outlet flange. The tapping machine shall be bolted securely to the valve.

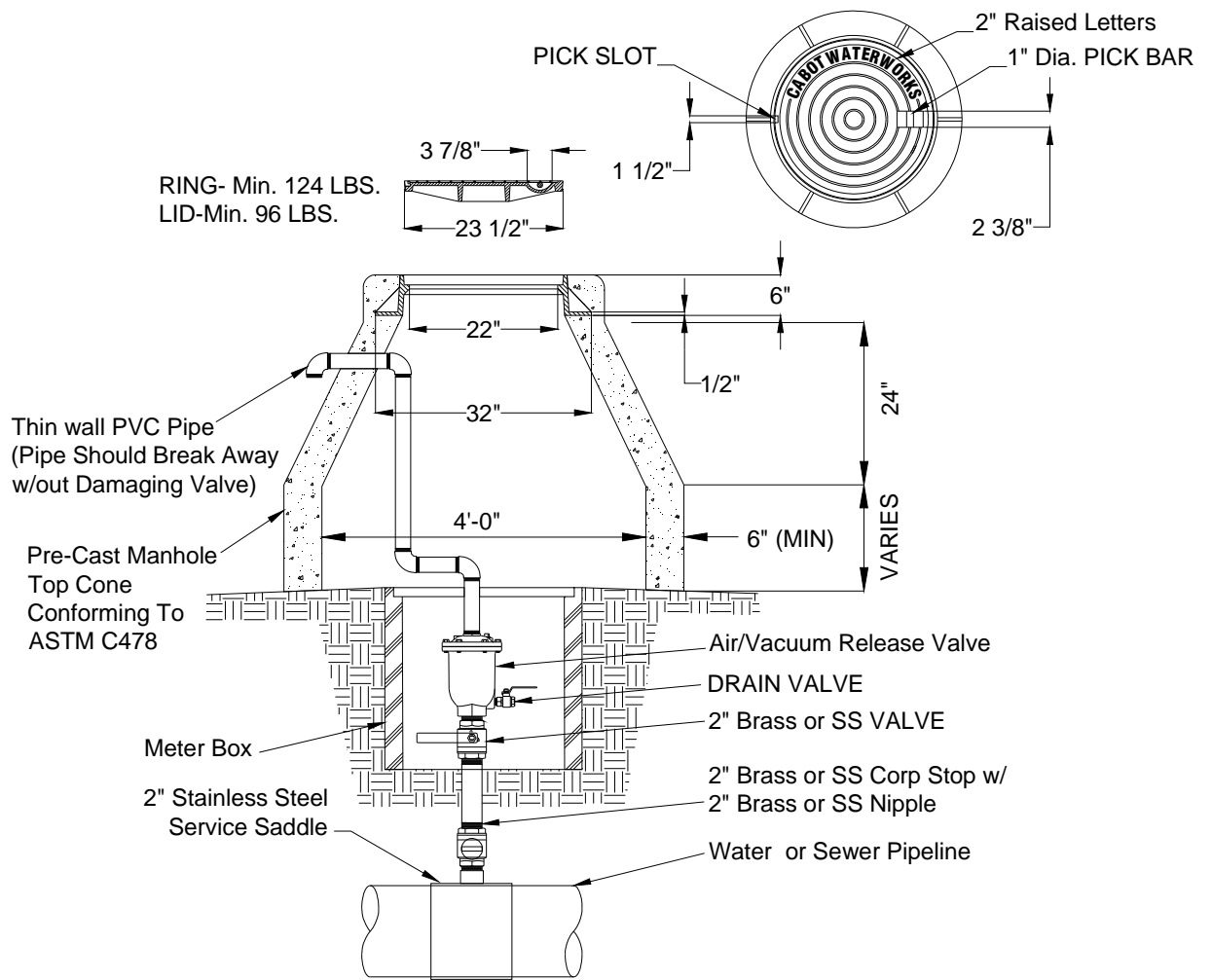
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- 18.6 After installation of the tapping sleeve or saddle and the tapping valve, and before drilling through the pipe, the sleeve or saddle shall be hydro-statically pressure tested at 150psi with an approved test pump connected to the test port on the sleeve or saddle.
- 18.7 The pilot drill and shell cutter shall be in good condition. The shell cutter shall be the size required to cut the full opening specified.
- 18.8 Only qualified operators shall operate the tapping machine. The "coupon" shall be withdrawn. Care shall be exercised to avoid drilling or cutting the back side of the pipe by carefully assuring the engagement of the pilot drill and shell cutter shaft.
- 18.9 Openings in the pipe barrel for tapping saddles installed on dry pipe shall be cut with a pilot drill and shell cutter. Torch cutting is not permitted.
- 18.10 Tapping operations must not commence before inspection by CWW' authorized representative.
- 18.11 A thick coat of bituminous material shall be applied to the straps and bolts of saddles after installation and cleaning or by enclosing polyethylene material in accordance **with Section 23 herein**. Coating or poly wrap will not be required if all metal surfaces are vinyl or epoxy coated and all bolts are stainless steel.

### **19. AIR/VACUUM RELIEF VALVES**

- 19.1 Air/Vacuum relief valves shall be constructed with a stainless steel or nylon body, and have a working pressure rating of 150 psi minimum. All interior operating parts shall be constructed of stainless steel. Air/Vacuum relief valves shall have an integral type assembly which functions both as an air release and vacuum relief valve.
- 19.2 Air /Vacuum relief valves shall be installed at high points along the pipeline as shown on the Plans.
- 19.3 Air/Vacuum relief valves shall be installed in accordance with the typical detail below. The inlet connection of air/vacuum relief valves shall be 2-inches in diameter. Service or tap saddles and pipe fittings used in connecting the air relief valve shall be Stainless steel conforming to ANSI/AWWA Standard C226 or Brass conforming to AWWA Standard C800. Connections on the outlet side of air/vacuum relief valves shall be threaded and shall have Stainless Steel or Brass elbows installed as indicated below.

**Cabot WaterWorks Standard Construction Specifications**



Typical Air/Vacuum Relief Valve

**20. STEEL PIPE for CREEK CROSSINGS**

- 20.1 Steel Pipe shall be used for Creek crossings as indicated on plans. Steel water pipe shall have minimum yield strength of 35,000-psi. The pipe shall conform to ASTM A-53 or ASTM A-135, Type E (electric resistance welded, Grade B) or Type S (seamless, Grade B). A letter of compliance from the Bidder or mill tickets shall be supplied to CWW for the delivered pipe.
- 20.2 Flanges for steel pipe creek crossings shall conform to AWWA Standard C207, class E, 275PSI.
- 20.3 Steel pipe shall not be used for direct burial installations. Ductile Iron pipe shall be used as a transition from direct burial to open creek crossings where steel pipe is to be used.
- 20.4 Maximum permissible spans for steel water pipe supported by AWWA C207, class E flanges are listed below. Steel water Pipe wall thickness shall be 3/8”.

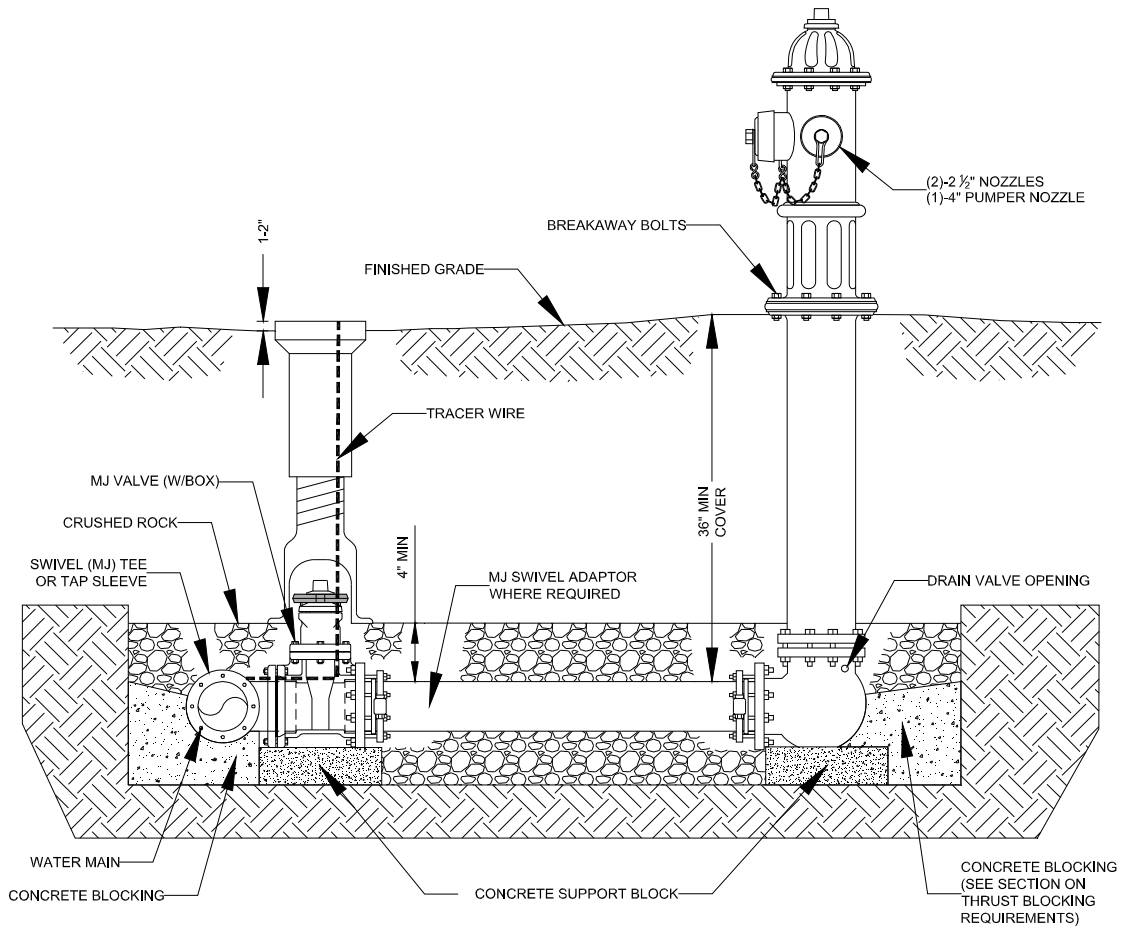
<u>Nominal Pipe Diameter (in.)</u>	<u>Wall Thickness (3/8”)</u>
8”	45
10”	46
12”	47

## Cabot WaterWorks Standard Construction Specifications

### 21. FIRE HYDRANT INSTALLATION

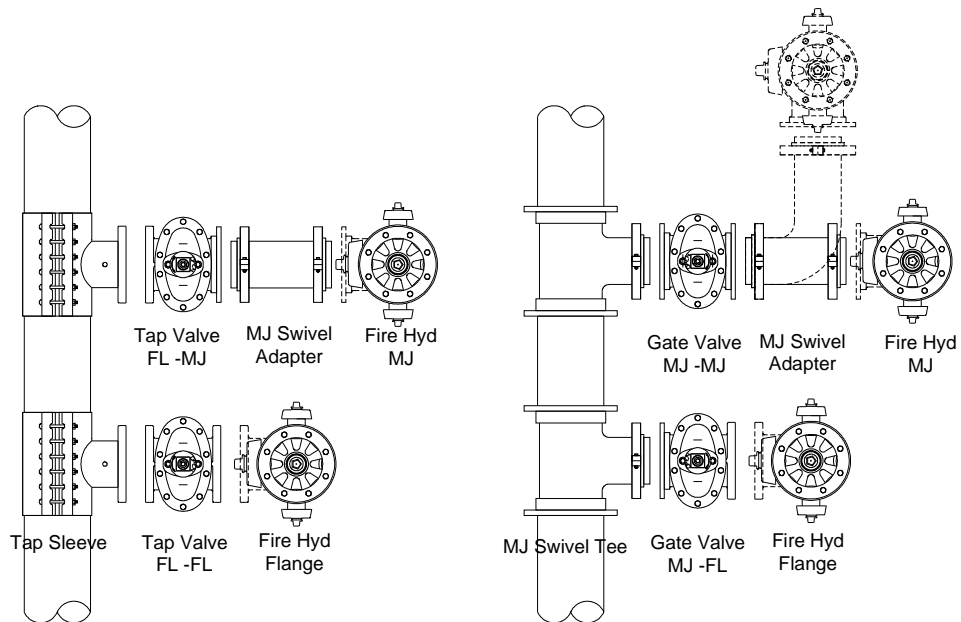
- 21.1 Remove all dirt and foreign matter from the hydrant barrel and bottom section up to the main valve prior to installation. The main valve shall be in the "closed" position. The drain outlet shall be cleared of obstructions.
- 21.2 Hydrants shall be installed fully restrained with MJ swivel Tee or swivel MJ adapters or flange to flange connections. Hydrants shall have a resilient seated gate valve installed on the hydrant lead from the main. Tapping valves used for hydrant lead connections, qualify as an auxiliary valve.
- 21.3 Hydrants shall be located a safe distance from driveways, roadways and sidewalks and in a manner to provide complete accessibility, and shall stand plumb with the hydrant base at finished grade.
- 21.4 Hydrants located on AHTD right-of-way shall be placed at the right-of-way line.
- 21.5 Unless otherwise directed, the pumper nozzle shall face the street. The CONTRACTOR shall, if necessary, rotate the hydrant barrel or nozzle section at the flanged joint to obtain the desired nozzle position as directed by the ENGINEER.
- 21.6 The base of the hydrant shall be supported and well braced against firm undisturbed soil. Stone slabs, concrete blocks, or other suitable material may be used to block the hydrant. As directed by the ENGINEER, the hydrant shall have concrete blocking placed around the hydrant foot, taking care to not cover the drain port with concrete.
- 21.7 A drainage bed shall be provided under and around the base of the hydrant and extending at least six (6") inches above the drain outlet and shall conform to Paragraph 10.8ii, Granular Bedding Material. Bedding shall be tamped around hydrant.
- 21.8 The hydrant drain outlet shall not be connected to a sewer.
- 21.9 New Hydrants shall be covered with plastic bags marked "Out-of -Service", until the hydrant is placed in-service.

# Cabot WaterWorks Standard Construction Specifications



NOTE: CONCRETE BLOCKING SHALL NOT COVER HYDRANT DRAIN VALVE OPENING.

## TYPICAL - FIRE HYDRANT ASSEMBLY NTS

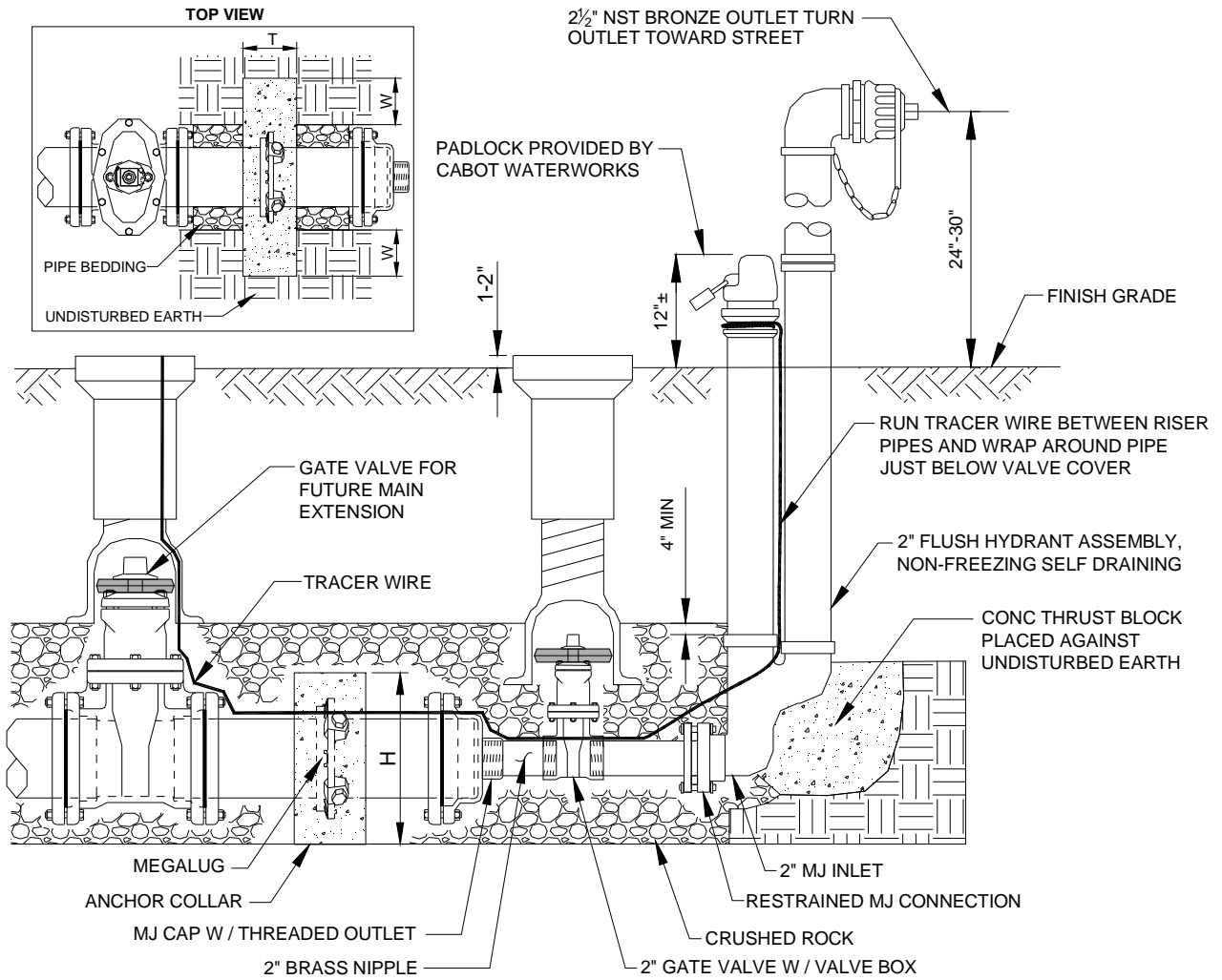


Typical Fire Hydrant Connection Detail

**Cabot WaterWorks Standard Construction Specifications**

**22. DEAD-END WATER MAINS w/ BLOW-OFFS**

22.1 Blow-offs shall be installed on all dead-end water mains in accordance with the detail below.



ANCHOR COLLAR SCHEDULE						
PIPE SIZE	* MINIMUM DIMENSIONS *			THRUST COLLAR	REINFORCING BARS	
	W	H	T		"A" BARS	"B" BARS
6"	1.5'	2.0'	1.0'	M.L. RETAINER GLAND	#6 @ 6"	#6 @ 6"
8"	1.5'	2.5'	1.0'	M.L. RETAINER GLAND	#6 @ 6"	#6 @ 6"
10"	2.0'	4.0'	1.5'	M.L. RETAINER GLAND	#6 @ 6"	#6 @ 6"
12"	2.0'	4.0'	1.5'	M.L. RETAINER GLAND	#6 @ 6"	#6 @ 6"

**NOTES:**

FLUSH HYDRANTS SHALL BE MAINGUARD NO. 77 AS MANUFACTURED BY KUPFERLE FOUNDRY CO., ST. LOUIS, MO, OR APPROVED EQUAL.

ALL WORKING PARTS SHALL BE BRONZE, AND BE SERVICEABLE FROM ABOVE GRADE. HYDRANTS SHALL BE LOCKABLE TO PREVENT UNAUTHORIZED USE

**DEAD-END WATER MAIN w/ BLOW-OFF ASSEMBLY**

**23. POLYETHYLENE ENCASEMENT for DUCTILE IRON PIPE**

- 23.1 All ductile iron pipe and appurtenances shall be completely encased in polyethylene tubing material. The polyethylene encasement shall be installed to prevent pipe-soil contact.
- a). Pipe-Shaped Appurtenances. Bends, reducers, offsets and other pipe-shaped appurtenances shall be covered with polyethylene in the same manner as the pipe.
  - b). Polyethylene material, either in tubing form or in the form of flat sheet or rolls, as specified herein, shall be placed around all mechanical joints of pipe and fittings; all valves and fire hydrants with mechanical joint ends and all saddles, sleeves, and couplings, tapping saddles and any other appurtenances with exposed bolts.
- 23.2 Polyethylene Encasement shall conform to AWWA C105.
- 23.3 Polyethylene material shall conform to ASTM D4976, virgin polyethylene. Sheets or rolls shall be either;
- a). 8 mil, linear low density with 3,600 psi tensile strength or
  - b). 4 mil, high density, cross laminated with 6,300 psi tensile strength.
- 23.4 Polyethylene tubing shall be applied to pipe by one of the following methods:
- a). Method "A"/ Cut polyethylene tube to a length approximately two (2') feet longer than length of the pipe section. Slip the tube around the pipe centering it to provide a one (1') foot over lap on each adjacent pipe section, and bunching it accordion fashion lengthwise until it clears the pipe ends. Lower the pipe into the trench and make up the pipe joint with the preceding section of pipe. A shallow bell hole must be made at joints to facilitate installation of the polyethylene tube. After assembling the pipe joint, take bunched polyethylene from the preceding length of pipe, slip it over the end of the new length of pipe and secure in place. Then slip the end of the polyethylene from the new pipe section over the end of the first wrap until it overlaps the bell joint at the end of the preceding length of pipe. Secure the overlap in place. Take up the slack width to make snug, but not tight, fit along the barrel of the pipe, securing the fold at quarter points with tape. See Ductile Iron Pipe Research Association (DIPRA), "Polyethylene Encasement Installation Guide" at <http://www.DIPRA.org>.
  - b). Method "B"/ Cut polyethylene tube to a length approximately one (1') foot shorter than the length of the pipe section. Slip the tube around the pipe, centering it to provide six (6") inches of bare pipe at each end. Make polyethylene snug, but not tight; secure ends. Before making up a joint, slip a three (3') foot length of polyethylene tube over the end of the preceding pipe section, bunching it in accordion fashion lengthwise. After completing the joint, pull the three (3') foot length of polyethylene over the joint, overlapping the polyethylene previously installed on each adjacent section of pipe by at least one (1') foot; make snug and secure each end.
- 23.5 Odd-Shaped Appurtenances. Valves, tees, crosses and other odd-shaped pieces which cannot practically be wrapped in a tube, shall be wrapped with a flat sheet or split length of polyethylene tube. The sheet shall be passed under the appurtenance and brought up around the body. Seams shall be made by bringing the edges together, folding over twice, and taping down. Slack width and overlaps at joints shall be handled as described in paragraph 23.4 above. Tape polyethylene securely in place at valve stem and other penetrations.
- 23.6 Openings in Tubing Material. Openings for branches, service taps, blow-offs, air valves, and similar appurtenances shall be made by making an x-shaped cut in the polyethylene and temporarily folding the film back. After the appurtenance is installed, tape the slack securely to the appurtenance and repair the cut, as well as any other damaged areas in the polyethylene with tape.
- 23.7 Junctions between Wrapped and Unwrapped Pipe. Where polyethylene wrapped pipe joins a pipe which is not wrapped, extend the polyethylene tube to cover the unwrapped pipe a distance of at least two (2') feet and secure the end.
- 23.8 The polyethylene material shall be secured around the pipe and appurtenances by at least three (3) circumferential wraps of tape (Section 9, Material Specifications).
- 23.9 All tongs, cables or chains that are used for lifting pipe and appurtenances that have been encased in polyethylene material shall be adequately padded to prevent damage to the material.
- 23.10 Repair any rips, punctures, or other damage to the polyethylene with tape or with a short length of polyethylene tube cut open wrapped around the pipe and secured in place.

## **Cabot WaterWorks Standard Construction Specifications**

- 23.11 Polyethylene material shall be stored on the job site in such a manner that it is not exposed to direct sunlight. Exposure during installation shall not exceed forty-eight (48) hours.
- 23.12 Backfill material shall be the same as specified for pipe without polyethylene wrapping. Special care shall be taken to prevent damage to the polyethylene wrapping when placing backfill. Backfill material shall be free from cinders, refuse, boulders, rocks, stones and/or other material that could damage polyethylene.

### **24. ENCASEMENT STRUCTURES (Open Cut or Bore & Jack)**

- 24.1 Encasement is required on all water and wastewater lines (including services) crossing Arkansas State Highways, and shall conform to the following requirements:
- a). Encasements shall be designed to support the load of the highway and superimposed loads thereon and, as a minimum, shall equal the structural requirements for highway drainage facilities. Encasements shall be composed of materials of satisfactory durability under conditions to which they may be subjected.
  - b). Encasements shall have a minimum cover of forty-eight (48) inches measured vertically from the flow line of parallel ditches or sixty (60) inches measured vertically from the highway surface; whichever provides the greater cover.
  - c). On non-controlled access highways, encasements should extend from right of way line to right of way line, as practical, and may be required by the District Engineer in some instances. As a minimum, encasements shall extend six (6) feet beyond the toe of the slope in any embankment section, the flow line of any parallel ditches, or back of any curb as applicable for the highway section.
  - d). Encasement shall be provided under medians and the area between frontage roads and the main lanes.
  - e). Encased crossings of partially controlled or fully controlled access highways shall be encased the full width of the control of access.
  - f). All crossings involving frontage roads shall be encased between the control of access and frontage road and the pipe shall extend a minimum of six (6) feet beyond the toe of the slope in any embankment section, the flow line of any parallel ditch, or the back of any curb as applicable on the side of the frontage road opposite the main lanes.
- 24.2 Casing Pipe installed for Sanitary protection of water pipe shall conform to all requirements of the Arkansas Department of Health, (see paragraph 5.1v)
- 24.3 The CONTRACTOR shall inspect the location where encasement structures are to be installed and become familiar with the conditions under which the work will be performed and with all necessary details as to the orderly prosecution of the work. The omission of any details in the Plans and Specifications for the satisfactory installation of the work in its entirety, which may not appear herein, shall not relieve the CONTRACTOR of full responsibility.
- 24.4 The CONTRACTOR shall satisfy himself of soil conditions by means he deems necessary; i.e., exploratory boring or exploratory pit excavations at tunnel ends. All such exploratory work shall be done in such a manner as to not jeopardize highway or railroad fill, and shall be satisfactorily back filled and the site restored.
- 24.5 The size structure as shown on the Plans is considered as the "minimum acceptable size". If the CONTRACTOR deems that it would be to his advantage to install a larger structure, he may do so subject to the approval of the ENGINEER. If the CONTRACTOR elects to use large structures, the unit bid price as stated in the Bid for the size structure specified will be considered the unit bid price for the larger structure. In no case will CWW be liable for extra payment for structures larger than the size specified.
- 24.6 Steel casing pipe shall have minimum yield strength of 35,000-psi and a wall thickness indicated in the Casing Pipe Sizing Table below. The pipe shall conform to ASTM A-53 or ASTM A-135, Type E (electric resistance welded, Grade B) or Type S (seamless, Grade B). A letter of compliance from the Bidder or mill tickets shall be supplied to CWW for the delivered pipe.
- 24.7 PVC casing pipe, where approved by the Engineer, may be used for open cut methods and shall conform to the polyvinyl chloride pipe DR 25.
- 24.8 Encasements shall be installed at the grades and alignment shown on the Plans. Variations shall be approved by the ENGINEER.
- 24.9 Where open cut is not practical or permitted, the encasement structure shall be installed by Boring and Jacking.

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- i. Boring and Jacking requirements:
  - a). Bore machine guide rails shall be carefully set in order to attain the specified grade, slope and alignment of the casing. Augers used shall match the outside diameter of the casing pipe specified. Over-reamed bore holes are not permitted. Any annular space between the casing and bore hole shall be filled by pressure grouting.
  - b). All bores shall be cased with steel casing
  - c). A two-inch auger pilot hole shall first be attempted to determine if rock will prevent the installation of the casing. If the pilot hole is successfully made, the casing shall be installed.
  - d). The leading section of casing shall be equipped with a jacking head securely anchored thereto to prevent any wobble or variation in alignment during the jacking operation.
  - e). Excavation shall be performed entirely within the jacking head and no excavation in advance thereof shall be permitted. Every effort shall be made to avoid any loss of earth outside the jacking head.
  - f). Excavated material shall be removed from the casing as excavation progresses, and no accumulation of such material within the casing will be permitted.
  - g). The boring and jacking operations shall be done simultaneously with correct line and grade carefully maintained for the casing. Holes for casing shall be bored with an auger mounted inside the pipe with the auger extending a short distance beyond the lead end of the pipe to preclude caving.

24.10 Pits and trenches shall be sheeted and braced in accordance with Section 9 herein.

24.11 The excavation and backfill of encasement pipe installed in open cuts shall be in accordance with the provisions contained elsewhere herein for other pipe.

24.12 Smooth wall steel pipe shall be connected by full penetration butt welding.

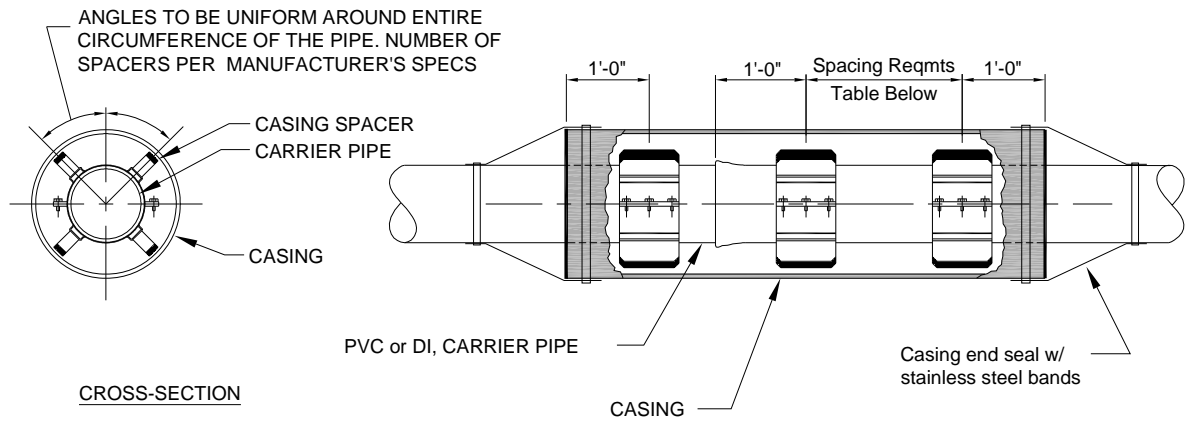
24.13 Steel Encasement pipe shall meet the minimum diameter and wall thickness in the table below for the pipe diameter being encased.

24.14 Carrier pipe shall be installed in a manner to assure that the joints are not over deflected or pulled apart during the process. The pipe shall be jointed and pulled or jacked through the encasement structure with a cable capable of pulling the total number of pipe lengths and shall be threaded through the casing. The pipe shall be pulled into the casing one joint at a time by means of a slow steady pull on the cable and guided by hand to prevent damage to the pipe, and to prevent the pipe and casing spacers from rotating out of alignment. The cable shall be attached to a wood block (of sufficient size to take the strain) placed across the end of the pipe. After each joint of pipe is pulled into the casing, the cable shall be taken loose and threaded through the next joint of pipe to be added to the line. This process shall be repeated until the pipeline section is completely installed in the casing.

24.15 All carrier pipe installed in casing shall have casing spacers installed at intervals indicated in the table below. Casing spacers shall be constructed of a Stainless Steel band with runners designed to support the filled pipe weight. If, after installation of the pipe, adequate stability has not been provided, in the opinion of the ENGINEER, the pipe shall be removed or the annular spaces between the pipe and encasement structure be filled with grout or sand, as required by the ENGINEER. Carrier pipe shall not be bear on bell joints in casing.

24.16 Ends of Encasement shall be sealed using a non-shrink grout or neoprene molded rubber end seal with stainless steel banding around casing and carrier pipe. Casing End seals shall be manufactured by Advance Products & Systems Model AC or Model AM or equal. End seals shall be approved by the Engineer.

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Casing Pipe sizing table

<u>Nominal Pipe Diameter (in.)</u>	<u>Casing Size Inside Dia. (in.)</u>	<u>Steel Casing Wall Thickness (in.)</u>	<u>Casing Spacer Maximum Spacing (ft.)</u>
4"	8 - 10"	0.25"	6.7' & each side of bell joint
6"	10 - 12"	0.25"	8.3' & each side of bell joint
8"	14 - 16"	0.3125"	10.0' & each side of bell joint
10"	16 - 18"	0.3125"	10.0' & each side of bell joint
12"	18 - 20"	0.375"	10.0' & each side of bell joint
14 - 15"	24 - 26"	0.375"	10.0' & each side of bell joint
16"	28 - 30"	0.375"	10.0' & each side of bell joint
18"	30 - 32"	0.375"	10.0' & each side of bell joint
20 - 21"	32 - 34"	0.375"	10.0' & each side of bell joint
24"	36 - 38"	0.375"	10.0' & each side of bell joint

### **25. CONCRETE THRUST BLOCKS & ANCHOR COLLARS FOR PIPE FITTINGS**

- 25.1 Concrete thrust blocks and anchors shall be provided along the pipeline in accordance with the **Typical Thrust Blocking Details** below, or as directed by the ENGINEER.
- 25.2 Concrete shall conform to requirements of Section 26, "Concrete Mix – Placement & Finishing" of these specifications. Cold weather requirements for concrete in paragraph 26.25 shall apply for thrust blocking.
- 25.3 **Dry** "Quick Crete" concrete will not be allowed for concrete blocking. "Quick Crete" concrete may be used for blocking if mixed with water per manufacturers recommendations.
- 25.4 Concrete for thrust blocks shall be placed against undisturbed soil. The excavation shall be hand shaped and free of loose material. Forms shall be used to confine the concrete in areas other than that part that is in contact with undisturbed soil in the direction of the thrust.
- 25.5 Concrete shall not be placed where it interferes with the removal of any joint accessories such as bolts, followers, threads, collars, couplings, etc. Fire hydrant drains shall not be restricted.
- 25.6 The top of the concrete thrust block or anchor collar shall be struck off with a wood straight edge or float.
- 25.7 All placement of concrete must be in the presence of the ENGINEER or his representative. The CONTRACTOR is cautioned that he may be required to remove, without compensation, any concrete placed in the absence of the ENGINEER or his representative.
- 25.8 Backfill over concrete thrust blocks or anchor collars shall not be placed before the concrete has attained initial set.
- 25.9 No thrust blocks shall be less than six (6") inches thick between the pipeline or appurtenances and undisturbed soil in the direction of thrust
- 25.10 The excavation shall be free of water before concrete is placed. Steel reinforcement, as specified on the plans, shall be placed in accordance with paragraph 26.19 herein.
- 25.11 The pipe or appurtenances shall be cleaned before placing concrete when the concrete is to be in direct contact with the pipe or appurtenance.

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25.12 The area of contact of the thrust blocks and anchor collars shall be sufficient to resist the thrust. This will vary depending on the safe bearing value of the soil. Suggested safe soil bearing values are as follows:

<u>TYPE OF SOIL</u>	<u>Estimated Bearing Capacity</u> (lbs./sq.ft.)
Muck, Peat, etc.	0
Soft clay	500
Sand	1000
Sand and Gravel	1500
Sand and Gravel with Clay	2000
Sand and Gravel cemented with Clay	4000
Hard pan	5000

25.13 The above values are approximate and will vary considerably and are intended to be used only as a guide. The **CONTRACTOR** is responsible for determining the soil bearing value or taking other action to assure that the bearing area is adequate to restrain the pipe or appurtenances.

25.14 Where the soil is unstable or in the case of recent fill areas, the following procedures shall apply either singly or in a combination:

- i. Thrust blocks shall be of adequate size to restrain pipe or appurtenances by mass alone without depending on horizontal bearing of the soil.
- ii. The excavation shall extend deep enough to contact firm soil and the block brought up to the pipe or appurtenances and constructed so that the block acts as a beam and will provide restraint required. Such block shall be reinforced with steel reinforcing bars.
- iii. Anchor blocks shall be constructed in a firm soil and tie rods extended to the pipe or appurtenances.

25.15 Thrust blocks for vertical bends shall be adequate to resist the thrust by mass alone when the thrust is upward.

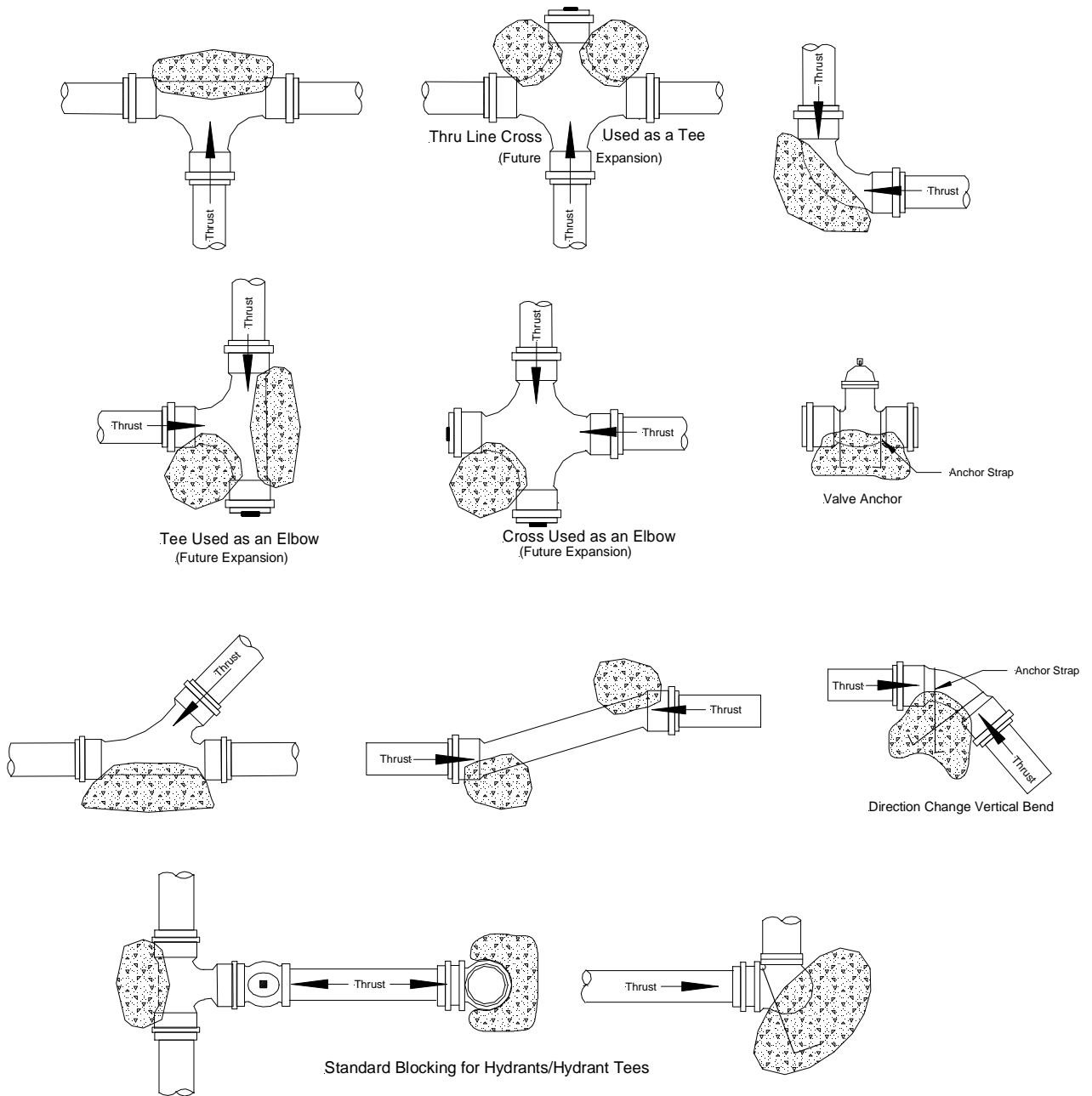
25.16 Thrust blocks and anchor collars shall be adequate to restrain the pipe appurtenances at the specified test pressure. The following table lists the resultant thrust at certain fittings at a pressure of 100 psi. In order to determine the thrust at the test pressure these values are to be multiplied by a factor equal to the test pressure divided by 100.

Thrust per 100 psi Pressure

<u>Fitting Thrust (lbs. of force)</u>	<u>4"</u>	<u>6"</u>	<u>8"</u>	<u>10"</u>	<u>12"</u>
22 1/2° Bend	600	1,200	2,000	3,300	4,400
45 °	1,100	2,300	4,100	6,300	9,100
90 °	1,800	4,000	7,200	11,200	16,000
Plug, Valves, Tees, Dead Ends	1,300	2,900	5,100	7,900	11,300

25.17 The projected soil bearing area required to resist the thrust developed in pipe bends and fittings shall be determined by dividing the pounds of thrust for the fitting size, from the table above, by the soil bearing capacity. The result of this computation will be square feet of projected area perpendicular to the direction of thrust as indicated in the figures below.

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### Typical Thrust Blocking Details

25.18 Concrete thrust blocks or anchor collars that fail to restrain the pipe or appurtenances shall be replaced by the CONTRACTOR at his expense.

## 26. CONCRETE MIX - PLACEMENT & FINISHING CONCRETE

26.1 Concrete materials shall conform to the following standards:

- ASTM C94 – “Standard Specification for Ready Mix Concrete”
- ASTM C150 – “Standard Specification for Portland Cement”, Type I

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- c). ASTM C33 – “Standard Specification for Concrete Aggregates”
  - d). ASTM C387 – “Standard Specification for Packaged, Dry, Combined Materials for Mortar and Concrete”
- 26.2 Class "A" concrete shall have a 28-day compressive strength of at least 4,000 psi. Class "A" Concrete shall be used for sidewalks, street surfaces, curb & gutters, ditch paving, driveways, structures such as vaults, manholes, protective slabs over pipelines and other facilities, footing pipe supports and anchor collars.
- 26.3 Class "B" concrete may be used for thrust blocking and shall have a 28-day compressive strength of at least 2,500 psi.
- 26.4 Only potable water shall be used for concrete mix.
- 26.5 Fine aggregate shall consist of clean, sound, sand conforming to ASTM Standard C33 meeting the following grading requirements:

<b>GRADATION FOR FINE AGGREGATE - ASTM C 33</b>		
<i>Sieve Designation (square openings)</i>		<i>Percentage by Weight Passing Sieves</i>
<i>in.</i>	<i>mm.</i>	
3/8 in.	9.5 mm	100
No. 4	4.75 mm	95–100
No. 8	2.36 mm	80–100
No. 16	1.18 mm	50–85
No. 30	600 micro-m	25–60
No. 50	300 micro-m	10–30
No. 100	150 micro-m	2–10

- 26.6 Coarse aggregate shall consist of crushed stone, gravel, or other inert material of similar characteristics, having clean, hard, strong, durable non-coated particles meeting the following grading requirements:

<b>Gradation for COARSE Aggregate - ASTM C 33</b>		
<i>Sieve Designation (square openings)</i>		<i>Percentage by Weight Passing Sieves</i>
<i>in.</i>		<i>%</i>
1		100
3/4		90–100
1/2		20–55
3/8		0-15
No. 4		0-5

- 26.7 In no case shall the maximum aggregate size exceed one-third (1/3) of the design thickness of any part of a structure.
- 26.8 Limits for deleterious substances in Fine and Course aggregates.

<b><u>Deleterious Material</u></b>	<b><u>Maximum percent by weight</u></b>
Clay Lumps, Friable particles & Chert	1.0 %
Material finer than # 200 sieve	1.0 %
Coal and Lignite	0.5 %

- 26.9 Aggregates for use in concrete shall not contain any materials that are deleteriously reactive with the alkalis in the cement. Aggregates shall be tested for potential alkali reactivity in accordance with ASTM Standard C289 and C227, test results shall be negative.

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### 26.10 Concrete Proportions and Consistency.

- a). Slump of concrete shall be between 2 inches and 4 inches as tested in accordance with ASTM C143.
- b). Air Content: 6%  $\pm$ 1.5% as tested in accordance with ASTM C231 for air-entrained concrete.
- c). Water-Cement Ratios shall be limited to 0.42 for all concrete unless otherwise specified.

26.11 Mixing and Hauling Concrete. Concrete shall be mixed in a batch mixer of a type designed to insure uniform and thorough distribution of the materials throughout the mix. Its size shall be such as to produce a volume of concrete consistent with demands of each pour. It shall be equipped with a water metering device for accurate measurement of water. Charges of cement shall be by full sacks of cement; the use of fractional sacks, or reclaimed cement, will not be acceptable. All concrete shall be mixed for a period of not less than five (5) minutes after all materials, including the mixing water, have been placed in the drum. The entire contents of each batch shall be discharged from the drum before any material for the succeeding batch is placed therein. When the mixing operation for each period is completed, the mixer drum shall be cleaned thoroughly. Concrete shall be deposited within forty-five (45) minutes after it is mixed.

26.12 If the CONTRACTOR elects to use a central mixing plant, he shall provide sufficient hauling equipment, properly designed to prevent segregation and loss of mortar, and to permit discharge without segregation. When the hauling and placing operation for each period is completed, the hauling equipment shall be cleaned thoroughly.

26.13 The CONTRACTOR shall obtain from the supplier of ready-mixed concrete, the supplier's agreement to inspection by the ENGINEER, to the full extent deemed necessary by the ENGINEER.

26.14 TESTING CONCRETE: At the direction of the ENGINEER, the Contractor shall collect samples of concrete for testing as specified below.

- a). Contractor shall collect concrete compression test specimen in accordance with ASTM Standard C31.
- b). Contractor shall have concrete specimens tested for compressive strength in accordance with ASTM Standard C39.

26.15 All reinforcing bars shall be "Billet-Steel Concrete Reinforcement Bars" conforming to ASTM Designation A15 or "Rail-Steel Concrete Reinforcement Bars" conforming to ASTM Designation A16. Billet-Steel bars shall be intermediate grade with minimum yield point of 40,000 psi

26.16 All reinforcing bars shall be deformed bars. Deformation shall comply with "Minimum Requirements of the Deformation of Deformed Steel Bars for Concrete Reinforcement - ASTM Designation A305."

26.17 When the volume of concrete required at the work site is less than one-third (1) cubic yard, mixing may be accomplished by hand tool methods. The concrete shall be mixed in a clean, water-tight vessel to the extent necessary to assure that the cement, aggregate and water are thoroughly integrated. The concrete mix shall be QUIKRETE Concrete Mix #1101 conforming to ASTM C387 – "Standard Specification for Packaged, Dry, Combined Materials for Mortar and Concrete" with a Only that amount of water required to provide a stiff, workable mix shall be used. Hand mixed concrete shall obtain the strength requirements specified above for Class "A" and "B" concrete.

### 26.18 PLACEMENT & FINISHING CLASS "A" CONCRETE

- i. Forms shall be constructed before placement of any concrete unless otherwise authorized by the ENGINEER.
- ii. Forms shall conform to shapes, lines, and dimensions of the members as specified on the Plans, or as required to conform to the original shape and dimensions in the case of replacement structures, and shall be sufficiently tight to prevent leakage of mortar. They shall be properly braced or tied together so as to maintain position and shape. They may be constructed of any material with sufficient strength which will provide the finished work a satisfactory surface; however, metal forms will not be permitted for concrete which will be exposed on any of the completed work, except upon the specific approval of the ENGINEER.
- iii. In those cases in which paved surfaces are being replaced such as sidewalks, driveways, curb, gutter, etc., the edge of adjacent existing surfaces must be cut straight and smooth using a concrete saw or

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- other suitable tool. The forms shall be constructed in such manner that a smooth concrete surface is produced which matches the existing surfaces on each side.
- iv. Forms shall be removed in such a manner as to insure the complete safety of the structure. When the structure is supported on shores, the removable floor forms, beams and girder sides, and column and similar vertical forms may be removed after 96 hours, providing the concrete will not be injured. In no case shall supporting forms or shoring be removed until members have acquired sufficient strength to support their weight and imposed loads safely.
  - v. Forms shall be coated with oil before placement of reinforcing steel or concrete. Excessive coating material shall not be allowed to form or stand in puddles in the forms nor allowed to come in contact with concrete against which fresh concrete will be placed.

### 26.19 Placement of Reinforcement.

- i. At the time concrete is placed, metal reinforcement shall be free from rust scale or other coatings that will destroy or reduce the bond. All bars shall be shop bent, unless otherwise permitted by the ENGINEER. No bars partially embedded in concrete shall be field bent except as shown on Plans or as specifically permitted by the ENGINEER. Field bonding of rail steel bars will not be permitted.
- ii. Metal reinforcement shall be accurately placed according to the Plans or as specified herein and adequately secured in position by concrete, metal, or other approved chairs, spacers or ties.
- iii. No splices of reinforcement shall be made except as shown on the Plans, or as specified, or as authorized by the ENGINEER. All welding shall conform to the American Welding Society's Recommended Practices for Welding Reinforcing Steel, Metal Inserts and Connections in Reinforced Concrete Construction (AWS D12.1), unless otherwise authorized by the ENGINEER. All laps and splices shall be in accordance with ACI 318, using  $f_c = 3000$  psi and  $f_y = 60,000$  psi unless otherwise shown on the Plans.
- iv. The reinforcement shall be protected by the thickness of concrete indicated in the Plans. Where not otherwise shown, the thickness of concrete over the reinforcement shall be as follows:
  - a). Where concrete is deposited against the ground without the use of forms, not less than 3-inches, except wire mesh reinforcement for concrete slabs which may be within 1 ½ inches of the ground.
  - b). Where concrete is to be exposed to the weather or to the ground but placed in forms, not less than 2-inches for bars larger than No. 5 and 1 ½ inches for No. 5 bars or smaller.
  - c). In slabs and walls not exposed to the ground or to the weather - not less than ¾ -inch.
  - d). In all cases - at least equal to the diameter of the bars.
- v. Reinforcement for Curb and Gutter. Reinforcement shall consist of #3 bars spaced at 6 inch centers longitudinally with #4 transverse tie bars spaced at 2-foot centers. The bars shall be firmly held in place by wiring bars together. Bar chairs or other suitable steel devices shall be used to support the bars at a level 3-inches above the bottom of the concrete.
- vi. Reinforcement for Sidewalk, Ditch Paving, Slope Protection and Driveway Surfaces. Reinforcement shall consist of 6-inch x 6-inch mesh, #6 for driveways and #9 for ditch paving, slope protection and sidewalks.

### 26.20 Joints and Joint Filler

- i. Water stops shall be installed at no additional cost to CWW for construction joints in walls and slabs below finished ground elevation, and in all water bearing structures. Where a joint is to be made, the surface of the concrete shall be thoroughly cleaned and all laitance removed. In addition, vertical joints shall be thoroughly wetted and flushed with a coat of neat cement grout immediately before placement of new concrete.
- ii. Expansion and contraction joints shall be for replacement sidewalk, driveway, curb and gutter, ditch paving, protective slabs; joints shall be spaced every 12 feet.
- iii. Joint Filler.

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- a). Pre-molded joint filler shall be the non-extruding type, 3/4-inch to 1 inch thick, conforming to ASTM Designation M33, and shall be of sufficient dimension to extend through the full depth of the paved surface.
- b). Poured joint filler shall be bituminous material conforming to ASTM Designation M18 or rubber-based compound conforming to Federal Specification SS-F-336.
- iv. Embedded items, wall fittings and anchor bolts shall be installed as shown on the Plans. Embedded items shall be worked-out in advance of the concrete pour, giving the ENGINEER ample time for inspection of all items to be embedded.

### 26.21 Placing Concrete.

- i. **All placement of concrete must be in the presence of the ENGINEER or his representative. The CONTRACTOR is cautioned that he may be required to remove, without compensation, any concrete placed in the absence of the ENGINEER or his representative.**
- ii. Equipment for chuting, pumping and pneumatically conveying concrete shall be of such size and design as to ensure a practically continuous flow of concrete at the delivery end without separation of materials. All concrete in walls and columns shall be poured through tremies unless otherwise permitted by the ENGINEER. The free fall of concrete shall be held to a minimum.
- iii. Concrete shall be deposited as nearly as practicable in its final position to avoid segregation due to re-handling or flowing. The placing of concrete shall be carried on at such a rate that concrete is at all times plastic and flows readily into the spaces between the bars. Concrete that has been contaminated by foreign material shall not be used, nor shall re-tempered concrete be used.
- iv. When placing concrete is started, it shall be carried on as a continuous operation until placement of the panel or section is complete. When construction joints are necessary, they shall be made in accordance with **Joints and Joint Filler herein**.
- v. In placing concrete, care shall be taken that the freshly placed mass is so placed and vibrated that there is no tendency for the coarse aggregate to segregate from the mortar, that no rock pockets are left, that the concrete flows readily around the steel and into the extremities of the forms, and the whole freshly-placed mass becomes a plastic jelly-like mass but without free water in appreciable quantity on its surface. Class "A" concrete shall be poured with mechanical or magnetic internal vibrators.
- vi. Water shall be removed from place of deposit before concrete is placed unless otherwise permitted by the ENGINEER. Before depositing concrete on or against concrete which has taken its initial set, the surface of the hardened concrete shall be broken off down to coarse aggregate and wire brushed to remove foreign matter and laitance. A layer of grout of the same cement-sand ratio as the concrete without coarse aggregate shall be placed to a thickness of one (1) to two (2) inches on the brushed surface after which the new concrete shall be placed immediately.
- vii. Concrete placed directly on the ground shall be placed in the forms on a compacted wetted sub-grade and shall be tamped and spaded until mortar covers the entire surface. Tamping and spading shall be given special attention in order to prevent voids in the concrete.
- viii. Curb and Gutter Section. The cross section of the curb and gutter shall conform to that of adjacent sections or as directed by the ENGINEER. The surface shall be struck off smooth between templates, shaped to the required cross section, and it shall be finished with a wooden float and steel trowel and a broom to produce a uniform finish to match that of adjacent sections. The curb and gutter shall be grooved with an approved grooving tool at intervals equal to those of adjacent sections where applicable.
- ix. Flat Surfaces
  - a). After the concrete has been deposited, it shall be approximately leveled and struck off to such depth below the finish grade as required to place reinforcing mats or wire mesh, and shall be properly consolidated prior to placing the reinforcing mats or mesh, when applicable. Additional concrete shall

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be deposited and struck off to a depth above the finish grade that when properly consolidated, the surface shall conform to the line and grade required to match adjacent surfaces. The strike-off board shall be straight, free from warp, and shod on the striking surface with a steel strip. The strike-off board shall be moved forward with a combined longitudinal and transverse motion and without raising either end above the side forms during the strike-off process. A slight excess of material must be kept in front of the cutting edge at all times.

- b). Immediately following screeding and compaction operations, the slab surface shall be tested for trueness with a straight-edge. Use a wood float, darby, or bullfloat to eliminate high and low spots and to embed large aggregate. This shall be done in a manner to produce even, uniform surfaces so that surface irregularities do not exceed 1/8 inch in 10 feet. The straight-edge testing and floating operation shall continue until the entire surface is found to be free from observable departures from the straight edge, and until the surface of the slab has the required grade and contour.

### 26.22 Concrete Finishing.

- a). The concrete shall be worked with suitable tools until a finish which matches that of adjacent surfaces is achieved.
- b). Exposed aggregate finish. Scrubbed finish shall be produced on green concrete. The surface shall be thoroughly wetted and scrubbed with stiff fiber or wire brushed, using water freely, until the surface film of mortar is removed and the aggregate is uniformly exposed. The surface shall then be rinsed with clean water. If portions of the surface have become too hard to scrub in equal relief, dilute hydrochloric acid (commercial muriatic acid diluted with 4 to 10 parts water) shall be used after the concrete is at least two (2) weeks old to facilitate the scrubbing. The acid shall be removed within fifteen (15) minutes from the finished surface with clean water. This operation may be facilitated by casting the concrete against form faces which have been coated with a chemical retarder to keep the mortar adjacent to the form from setting. Every effort must be exerted to assure that the new surface matches any existing adjacent surfaces.
- c). All exposed inner and outer (above grade) walls of structures shall be finished as follows: Forms shall be removed from such surface as soon as structurally possible and when approved by the ENGINEER. All imperfections and depressions shall then be immediately filled with mortar. The concrete surfaces shall then be machine or hand rubbed until the entire surface has a smooth, homogenous, pleasant-appearing finish of uniform texture and color. Any delay in patching or rubbing such surfaces shall be cause for rejection of the entire structure. No mortar or wet cement shall be used in finishing except the mortar necessary to fill imperfections. Edging tools shall be used on all exposed joints and edges. All exposed corners shall be chamfered as shown on the Plans.
- d). All other wall surfaces of structures shall be finished as follows: Forms shall be removed from such surfaces as specified above. All imperfections and depressions shall then be immediately filled with mortar and rubbed with a wooden float or steel trowel to give a uniform appearance. Spurs shall be removed from the concrete surface. No over-all machine or hand rubbing is required.
- e). Floors of structures and ditch pavement shall be trowel finished.

26.23 Concrete shall be protected from damage while curing. Provisions shall be made for maintaining concrete in moist condition for a period of at least ten (10) days after placement.

26.24 After removal of forms and finishing, as specified elsewhere herein, backfill shall be placed around the structure and thoroughly compacted.

### 26.25 Cold-Weather Requirements.

- a). Concrete shall not be placed when the ambient temperature is below 40°F, or when the concrete is likely to be subjected to freezing temperatures before final set has occurred. Concrete footings or slabs shall not be placed over frozen ground. The temperatures of the concrete when placed shall not be less than 45°F. Heated materials shall be free of ice, snow and frozen lumps before entering the mixer. Methods and equipment for the heating of materials shall be subject to the ENGINEER'S approval. Suitable means shall be provided for maintaining the concrete at a temperature of at least 45°F for not less than 96 hours after placing.
- b). Special methods shall be used to protect concrete slabs in from freezing after concrete placement. Methods shall include covering of slabs with hay or other insulating material, protection of insulation from wetting with tarpaulins, introduction of heat, construction of enclosures, or other methods,

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depending upon the severity of the weather. The housing, covering or other protection used in connection with curing shall remain in place and intact at least 24 hours after the artificial heating is discontinued. Methods to be used shall be proposed in advance of the work and shall be approved by the ENGINEER. Salt, calcium, chloride, or other chemicals shall not be mixed with the concrete to prevent freezing.

- c). Any and all concrete damaged by freezing shall be removed to the satisfaction of the ENGINEER, and replaced, all at the expense of the CONTRACTOR.

### 26.26 Hot Weather Requirements.

- i. In hot weather, suitable precautions shall be taken to avoid drying of the concrete prior to finishing operations. Use of windbreaks, sunshades, fog sprays, or other devices shall be provided as directed by the ENGINEER.
- ii. Concrete deposited in hot weather shall not have a placing temperature that will cause difficulty from loss of slump, flash set, or cold joints. Concrete temperatures shall be less than 90°F unless higher temperatures are permitted by the ENGINEER.
- iii. The use of curing compounds or other additives shall be prohibited unless approval of its use has been secured, in advance, from the ENGINEER.

## 27. **REPLACEMENT of PAVED SURFACES**

27.1 Paved Surfaces shall mean Portland cement concrete and/or bituminous asphalt pavement

27.2 The CONTRACTOR shall replace or repair all paved surfaces removed or disturbed during the progress of work.

27.3 All replacement or repairs of paved surfaces within the right-of-way of public streets or roads must conform to requirements of and be approved by the state, county or local authority having jurisdiction. Concrete surfaces, except street pavement, shall be constructed in accordance with **Section 26**, unless otherwise directed by state, county or local authorities having jurisdiction. Street repairs shall conform to the detail for Type 5 Bedding; see **Section 10 – Pipe Bedding and Backfill**.

27.4 The materials used shall be of the same type as that removed or materials approved by the ENGINEER or state, county, or local authority having jurisdiction.

27.5 The following shall be considered minimum requirements with respect to replacing asphaltic concrete pavement other than Public Street or alley pavement.

- i. The pavement thickness shall be at least as thick as adjacent pavement but in no case shall it be less than two (2") inches thick.
- ii. Before placement of new surface material, all excess material (temporary cold mix asphaltic concrete, crushed stone, soil, etc.) shall be removed to a depth at least equal to the specified thickness of the replacement surface. The base surface on which the replacement material is to be placed shall be finished smooth and any wet material and debris removed.
- iii. Preparation and Priming. No bituminous material shall be placed upon a surface having excess moisture or when general weather conditions are not suitable.
- iv. The edges of existing pavement surface shall be cut smooth and vertical along a horizontal straight line. All saw cuts shall be square with existing pavement.
- v. Remove all saw cut debris from the repair area. The base surface shall be finished smooth and rolled with a flat wheel, pneumatic tired roller or other compaction equipment to compact the uppermost portion of the repair area. Paved areas adjacent to the repair shall be cleaned by broom sweeping. The prime coat shall be applied at a rate of 0.40 gallons per square yard. The vertical edges of existing pavement shall be primed.

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- vi. Bituminous asphalt shall be transported to the work site in vehicles with a cover over the cargo compartment to retard heat loss. The mixture shall be delivered at a temperature of 275°F to 325°F. The mixture shall be spread by hand or suitable machine immediately after delivery to the work site to a depth which will match adjacent pavement surfaces after rolling. The mixture shall be thoroughly compacted by passing a steel wheel roller over the surface course immediately after being spread. The mixture shall be continuously rolled until all roller marks are eliminated and no further compaction is possible. The final surface shall match adjacent pavement surfaces and there shall not be any depressions or raised surfaces across the repaired areas.
- vii. All preparatory work must be inspected by the ENGINEER before placement of asphaltic concrete.

### **28. TEMPORARY REPAIRS to PAVED SURFACES**

- 28.1 Unless permanent repairs to paved street surfaces can be started within two (2) days after back filling the trench, the CONTRACTOR shall place suitable asphalt material in the trench. The surface shall match the surfaces on each side of the trench and shall be in accordance with applicable state, county, or local requirements.
- 28.2 The above provisions shall apply to driveways, sidewalks, parking lots, or other paved areas whenever the backfill will not adequately support vehicular traffic, whenever dust from the trench creates a nuisance, or whenever the trench is a hazard.
- 28.3 Detours shall have a gravel, crushed stone, or asphalt surface. Dust shall be controlled by the application of asphalt or water.
- 28.4 The CONTRACTOR shall maintain all temporary surfaces in good condition until permanent repairs are complete.

### **29. CONNECTIONS TO CWW DISTRIBUTION and COLLECTION SYSTEM**

- 29.1 All connections to CWW existing distribution and collection system must be accomplished in the presence of the ENGINEER or his representative.
- 29.2 In cases where completing the connection will disrupt service to customers, the CONTRACTOR shall notify the ENGINEER at least two (2) days in advance of the work. The customers whose service will be disrupted shall be notified by the CONTRACTOR. The CONTRACTOR shall plan the work so that disruption of service is held to a minimum. The plan must be satisfactory to the ENGINEER.
- 29.3 After connections have been completed, the valves shall be tightly closed.

### **30. FILLING WATER PIPELINES**

- 30.1 After the pipelines and appurtenances have been installed, all concrete thrust blocking has cured adequately and upon approval of the ENGINEER, the pipelines shall be filled with water.
- 30.2 In order to prevent circulation of water through the new pipelines back into the distribution system, only one valve shall be opened to allow water to flow into the new pipelines. This valve will be tightly closed after the filling operation has been completed.
- 30.3 The valve operated to fill the pipelines shall be operated slowly and shall not be fully opened. The valve shall be opened only enough to hear water flowing into the pipeline. This is determined by listening to the valve key after each full turn of the key. The operation of the valve shall be under the direction of the CWW Representative.
- 30.4 All air shall be expelled from the pipeline by opening fire hydrants, service lines and/or other openings installed at the pipeline crests by the CONTRACTOR. The location and number of such openings shall be as shown on the Plans or as directed by the ENGINEER.

**31. CLEANING LARGE WATER PIPELINES**

- 31.1 Before disinfection, hydrostatic pressure and leakage testing (Section 32), pipelines 16-inches in diameter and larger shall be cleaned by forcing a resilient high density Polyurethane foam "cleaning pig" through each segment of the pipeline by water pressure to remove any dirt or other foreign matter from the pipeline. The "pig" shall be Blue Criss-Cross Polly Pig, manufactured by Knapp Polly Pig, Inc, Houston, Texas, or equal, in good condition and shall be at least two (2%) percent larger in diameter than the inside diameter of the pipeline being cleaned.
- 31.2 The "pig" shall be inserted in the first length of pipe installed in each segment of pipeline. After installation of the pipeline segment to be cleaned, the pipeline shall be filled at a point downstream of the "pig". The "pig" shall be forced through the pipeline by applying water pressure to the rear of the "pig" and opening blow-off valves or fire hydrants downstream of the "pig". Valves at blow-off points shall be closed immediately prior to the "pig" passing.
- 31.3 Based on past experience, the "pig" will negotiate bends and tees in the pipeline and may also be forced through the pipeline backwards after a pass forward. Openings in the pipeline, as shown on the Plans for removing the "pig", are based on this assumption. If the CONTRACTOR is unable to clean the pipeline, utilizing the openings provided, additional openings shall be provided by the CONTRACTOR, upon approval of the ENGINEER, at the expense of the CONTRACTOR.
- 31.4 If the need for more than one pass of the "pig" through the pipeline is indicated, the CONTRACTOR shall make additional passes as directed by the ENGINEER.
- 31.5 Drainage at blow-off points shall be provided so as not to create a nuisance and to avoid property damage.

**32. HYDROSTATIC PRESSURE and LEAKAGE TESTS Water & Sewer Force Mains**

- 32.1 After the CONTRACTOR has satisfied himself that the pipeline will pass a pressure and leakage test the CONTRACTOR shall contact CWW to witness the test.
- 32.2 At the direction of CWW' representative, the Contractor shall isolate sections of the pipeline, fill with water from a low point and release trapped air at crests through fire hydrants, service lines, air/vacuum relief valves or other openings installed by CONTRACTOR.
- 32.3 The CONTRACTOR shall furnish all labor, materials and equipment for conducting the tests.
- 32.4 CONTRACTOR shall provide a water pressure test pump for pressure and leakage test with the following features:
  - a). Designed for hydrostatic pressure and leakage test so the required test pressures can be attained.
  - b). By-pass pipe between the pump suction and discharge. By-pass shall be equipped with an in-line valve and a valve on the discharge.
  - c). Check valve arranged so as to prevent flow back toward pump.
  - d). Adjustable pressure regulating device capable of maintaining discharge pressure at a constant level.
  - e). Section of flexible hose - length sufficient that ends of hose rests on ground.
  - f). Straight meter coupling - 1/2" M.I.P. x 5/8" Meter Nut.
  - g). 5/8" meter - furnished by owner.
  - h). Straight meter coupling - 1/2" M.I.P. x 5/8" Meter Nut.
  - i). Connection for pressure gauge with valve and surge dampening device. Connection for gauge shall be 1/4" F.I.P. **CWW will furnish the meter and pressure gauge.**
  - j). Suction screen to prohibit entry of foreign matter if pump suction is connected to a vessel instead of on a water main.
- 32.5 The CONTRACTOR shall provide all other necessary materials for connecting the test pump from the suction source and to the main being tested.
- 32.6 The test pressure shall be a minimum **150 psi or the rated pressure class of the pipe material as specified by CWW, whichever is greater.** All pipelines and appurtenances shall be tested.
- 32.7 The duration of the hydrostatic pressure test shall be **two (2) hours** or as specified by the ENGINEER.

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- 32.8 The source of water for the pump suction shall be potable water from CWW' distribution system. The vessel used must be approved by the ENGINEER.
- 32.9 All interior valves including valves on fire hydrants and other appurtenances shall be open during all tests.
- 32.10 After the specified test pressure has been applied, the entire pipeline shall be inspected in the presence of CWW' representative, particularly appurtenances that are exposed.
- 32.11 After the CONTRACTOR has taken the necessary action to repair or replace any part of the pipeline or appurtenances where leaks were apparent or if no leaks were apparent, the pipelines shall be subjected to a leakage test at the pressure specified with a meter inserted in the test pump discharge line.
- 32.12 At the end of the two hour pressure test the test pump shall be used to restore the pipeline pressure to the specified test pressure. Meter readings shall be taken before recharging the pipeline and after the pressure is restored to determine leakage. Alternatively to meter readings, the suction line of the test pump may be placed in a container with volume graduations to determine leakage amount. CWW shall approve container.

The maximum leakage per hour for ductile iron and PVC pipe shall be as calculated from the following formula:

$$Q = \frac{L \cdot D^2 \cdot \sqrt{P}}{148,000}$$

- Q = Allowable Leakage (gallons per hour)
- L = Length of pipeline tested (feet)
- D = Nominal diameter (inches)
- P = Test pressure (psi)

- a). For listed test pressures, the following table provides allowable leakage per 1000 feet of pipe in gallons per hour.

Nominal Pipe Dia Inches	Test Pressure -	
	150 psi	200 psi
	Allowable leakage per 1000 feet (gal/hour)	Allowable leakage per 1000 feet (gal/hour)
4	.33	.38
6	.5	.57
8	.66	.76
10	.83	.96
12	.99	1.15

- 32.13 When leaks are in excess of the above formula or table, the CONTRACTOR shall, at his own expense, perform whatever work and/or replace whatever material that is required in order to remedy the defect and stop the leaks. All corrective work shall be approved by CWW.
- 32.14 As directed by the Engineer, the Contractor shall perform follow-up pressure test on any lines that the Contractor has modified or disturbed after the initial pressure test.

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### 33. FLUSHING WATER PIPELINES

33.1 Prior to disinfecting, all new pipelines shall be flushed to remove particulate. The flushing velocity in the pipe shall not be less than 2.5 ft./sec. The table below provides the required flow and number of 2" blow-offs and/or 2.5" fire hydrant nozzles opened to achieve the required flow for flushing.

Pipe Diameter Inches	Flow required for 2.5 ft./sec velocity to Flush new Pipe (gpm)	Number of 2" Blow-offs and/or 2.5" Fire Hydrants opened to achieve 2.5 ft./sec.
4	100	1
6	220	1
8	400	1
10	600	2
12	900	2
16	1600	3

33.2 The Engineer shall approve all flushing plans. The ENGINEER may halt or reduce flushing if the distribution system pressures are reduced by the flushing operations.

33.3 Flushing of new pipelines shall continue until no particulate is visible in the flushing discharge stream.

### 34. DISINFECTING WATER PIPELINES & APPURTENANCES

34.1 Disinfecting Water Mains shall be performed in accordance with AWWA C651 and the requirements of this Section.

34.2 Blow-off and sample points shall be constructed by the CONTRACTOR, as shown on the Plans or as directed by the ENGINEER. Fire hydrants shall be utilized as blow-off points whenever possible. Fire hydrants are not satisfactory for sample points. Openings for sample points shall be 3/4" or 1" copper riser pipe with a valve that is suitable for collecting bacteriological samples. A flushing plan shall be so designed so that all samples may be taken on each trip. Pipelines shall not be considered acceptable until two (2) consecutive bacteriological samples taken twenty-four (24) hours apart are negative.

34.3 There are two acceptable methods of disinfecting water mains: the *continuous feed method*, and the *slug method*. The slug method shall be used only on approval of the ENGINEER.

a). **The "tablet method" specified in AWWA C651 is not permitted.**

b). Chlorine used for disinfection shall conform to ANSI/AWWA B300 or B301 and may be either liquid chlorine, sodium hypochlorite, and calcium hypochlorite.

34.4 **Water pipe and appurtenances shall be thoroughly flushed at 2.5 ft./sec velocity prior to disinfecting.** See Section 33 "FLUSHING WATER PIPELINES" for flushing requirements.

34.5 The CONTRACTOR is reminded that chlorine is a powerful oxidant and reacts readily with foreign substances. All chlorine compounds shall be handled and stored in accordance with manufacturer's recommendations. Breathing of chlorine gas can be fatal. Hypochlorite solutions should not come into contact with skin or clothing. Containers used for mixing hypochlorite solution shall be clean and dry.

34.6 Backflow devices shall be installed to prevent chlorine solutions from back-flowing into the distribution system. Cabot Waterworks shall approve all backflow devices.

34.7 When the *continuous flow method* is used, the final concentration of chlorine inside the main shall be 25 mg/L.

34.8 Calcium hypochlorite shall contain minimum 65% available chlorine by weight either in tabular or granular form.

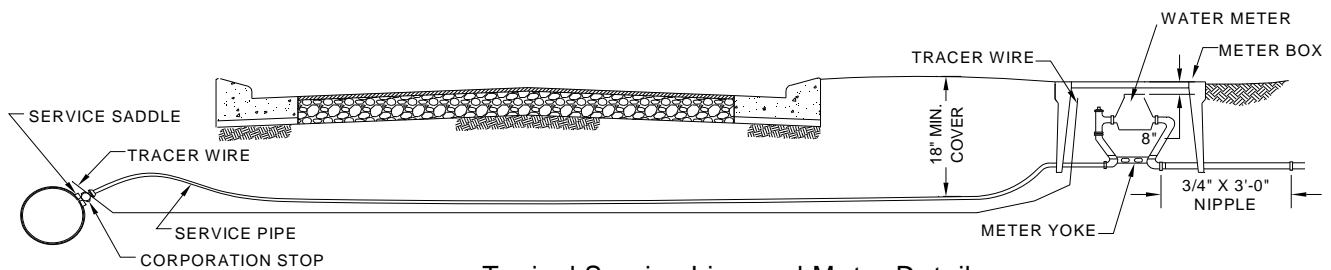
34.9 When the source of the chlorine is calcium hypochlorite, a solution of hypochlorite and water shall be prepared by mixing thoroughly in a suitable container. The mix shall contain one (1) pound of calcium hypochlorite per gallon of water. A suitable pump shall be provided for pumping this solution into the pipelines to be disinfected. This pump shall be equipped with a flow measuring device.

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- 34.10 Liquid chlorine shall be used only when the CONTRACTOR has suitable equipment available and employees who are familiar with the physiological, chemical and physical properties and who are properly trained and equipped to handle any emergency that may arise.
- i. Liquid chlorine shall not be used if in the opinion of the ENGINEER, the equipment is inadequate or the personnel are not qualified.
  - ii. When liquid chlorine is used, the equipment for injection shall consist of a solution feed chlorinator in combination with a booster pump for injecting the chlorine-gas and water solution into the pipeline.
  - iii. Introduction of chlorine-gas directly from the supply cylinder shall not be permitted.
  - iv. The booster pump shall be equipped with a flow measuring device.
- 34.11 The procedure for disinfecting by the *continuous flow method* shall be as follows:
- i. The flow through the pipeline and the solution flow shall be regulated so that the required concentration of chlorine is attained. The flow through the main shall be measured by using a pitot gauge or meter.
  - ii. The introduction of the solution shall be continuous until the desired concentration is attained throughout the pipeline system. The concentration shall be checked by the Drop Dilution Method.
  - iii. After the required concentration has been attained all internal valves shall be operated in order to assure that the solution comes in contact with all appurtenances.
  - iv. The solution shall remain in the pipeline system for twenty-four (24) hours after which the pipelines shall be thoroughly flushed. The chlorine concentration shall be checked before flushing. If the concentration is less than 25 mg/L, the disinfecting procedure shall be repeated if directed by the ENGINEER.
- 34.12 When the *slug method* is used, the concentration of chlorine entering the pipeline shall be at least 100 mg/L.
- i. The free available chlorine residual shall be measured as the slug moves through the pipeline. If the free available residual drops below 50 mg/l, the flow shall be stopped and additional chlorine added to restore the slug to 100 mg/l free available chlorine.
- 34.13 The procedure for disinfecting by the *slug method* shall be the same as the continuous flow method except that the flow rates shall be regulated so that the specified concentration of chlorine shall be in contact with all parts of the pipe for at least three hours.
- 34.14 After final flushing, samples of water shall be collected by CWW from the sample points provided by the CONTRACTOR. The CONTRACTOR shall provide any assistance required in collecting the samples.
- 34.15 Water samples collected shall be tested for bacterial contamination. Bacterial Tests on water samples shall be performed by the Arkansas Department of Health.
- 34.16 If the Engineer deems necessary, water contaminate testing in addition to bacteriological may be performed. If the pipe and appurtenances are found to be contaminated by the actions of the Contractor, the Contractor shall be responsible for all costs to remove the contamination or replace the pipe and appurtenances at his cost.
- 34.17 If any of the samples collected have positive test results for bacterial contamination, the disinfecting procedures shall be repeated as directed by the ENGINEER until negative samples are collected. Only the continuous flow or slug method may be used.
- 34.18 The cost of water used for flushing after positive samples have been collected shall be deducted from amounts due the CONTRACTOR. The amount used shall be determined by measurements of flow using a pitot gauge. Payment shall be based on the lowest prevailing water rate.
- 34.19 CONTRACTOR shall pay CWW **\$100.00 per sample** for each positive sample collected to cover the cost of collecting and delivery of samples to the Arkansas Department of Health.

**35. WATER SERVICE LINES, CONNECTIONS, and METERS**

- 35.1 Requirements of SECTION 1 of these specifications apply for service line installations.
- 35.2 Service connections shall be installed with service saddles designed for the diameter pipe to be tapped. Mains to be tapped shall be a wet tap using a corporation stop threaded into the service saddle. The service saddle shall be installed so that the pipe will not be distorted when the saddle is tightened.
- 35.3 Service saddles shall be positioned so that the corporation stop will be directed between 15° to 30° from the horizontal spring-line of the pipe.
- 35.4 The CONTRACTOR shall furnish a service tapping machine that permits a shell-type cutting tool to be feed through the corporation stop with the main under pressure to cut a hole in the pipe. The pipe coupon shall be retained by the shell cutter and removed from the cutting tool after each tap.
- 35.5 Service pipe shall be installed with a minimum of 18" of cover. Top of water meter shall be set 8"-10" below finish grade. Top of meter boxes shall be set 2" above finish grade.
- 35.6 Meter shall be located in a meter box between outside of curb and building lot lines. Meters shall not be placed on private property. Meter boxes shall be approved by Cabot WaterWorks.
- 35.7 Meter setters shall be supported with PVC pipe or rebar to keep the setter plumb.



Typical Service Line and Meter Detail

**36. WORK PERFORMED BY OWNER**

- 36.1 The intent of these specifications is for the CONTRACTOR to do all the work, including connections and repair of damage to the Distribution and Collection System in this construction contract.
- 36.2 In emergencies, to restore service, CWW, with its labor forces, will assist the CONTRACTOR when necessary at the discretion of the ENGINEER.
- 36.3 If CWW assists the CONTRACTOR for any reason, the CONTRACTOR shall pay for the cost of this assistance, based on the cost of labor, equipment, materials and overhead.

**37. DAMAGE TO DISTRIBUTION and COLLECTION SYSTEM**

- 37.1 The CONTRACTOR shall be held responsible for damage to CWW' distribution and collection system during the course of construction under this contract, whether by accident or carelessness. Contractor shall make immediate repairs to damages at his expense. In event service has been disrupted, immediate repair operations shall be continuous and around the clock, if necessary.
- 37.2 Disinfection procedures when cutting into or repairing existing water mains.
  - i. Disinfection procedures in this section are not required if repair clamps are installed while the pipe remains pressurized.
  - ii. The interior of all pipe and fittings, including couplings and sleeves shall be swabbed or sprayed with a one- percent hypochlorite solution before installation.

**38. SANITARY SEWER MANHOLES**

- 38.1 Brick, masonry, and vitrified clay block manholes will not be accepted.
- 38.2 Concrete used in the construction of cast-in-place and pre-cast manholes shall have a 28-day compressive strength of at least 4,000 psi conforming to ASTM C94, "Standard Specification for Ready-Mixed Concrete" or ASTM C387, "Standard Specification for Packaged, Dry, Combined Materials for Mortar and Concrete".
- 38.3 Concrete shall conform to requirements of Section 26, "Concrete Mix – Placement & Finishing" of these specifications. Cold weather requirements for concrete in paragraph 26.25 shall apply for manholes.
- 38.4 Pre-cast manholes shall conform to ASTM C478, "Standard Specification for Precast Reinforced Manholes". Pre-cast manhole joints shall be sealed using a butyl mastic sealant conforming to ASTM C990. Exterior Pre-cast manhole joints shall be wrapped with an 8" wide EPDM rubber, non-hardening butyl mastic seal, minimum 60 mil thickness to provide a water-tight seal around perimeter of joint. The interior annular space at horizontal joints shall be grouted after installation with a non shrink grout.
- 38.5 Hydraulic-Cement Grout (non-shrink) used for installing pipe in manholes shall conform to ASTM C1107. Non-shrink grout shall be specially formulated to stop infiltration. Grout that has not been used within 30 minutes after water has been added shall not be used.
- 38.6 Manhole Rings and Lids shall be a minimum of 250 pounds combined weight with a 23 ½" diameter lid. Manhole lids shall have 2 inch tall, raised letters reading "SANITARY SEWER" cast into the lid. Lids shall have two pick bars cast in the lid. Manhole lids with city names or any other lettering cast into the lid will not be accepted.
- 38.7 Stub-outs shall be constructed at the locations shown on the Plans. The stub-out shall be constructed in the direction and shall be the size and pipe material as shown on the Plans. The stub-out shall be bedded and backfilled as specified for sewer pipe used. After the stub-out installation is complete, watertight caps shall be installed on the end of the pipe. All stub-outs shall be pressure tested as specified for gravity sewers in these Specifications.
- 38.8 Pipe Connections to Manholes shall be made with a manhole adapter gasket conforming to ASTM C923 and designed to connect PVC sewer pipe to a concrete manhole wall. Manhole Adapter Gaskets shall be equal to Romac Style LCT or FERNCO Inc. and grouted or cast into manhole openings. Connections to Pre-cast manholes may optionally be made with Kor-N-Seal Pipe-to-Manhole Connectors or equal. All manhole connectors shall be installed in accordance with manufacturer's recommendations. All interior annular space between pipe and manhole opening shall be filled with non-shrink grout conforming ASTM C1107.

**EXECUTION:**

- 38.9 Excavation for Manholes. Excavation for manholes will be made of such dimension and depth as to allow the construction of the manhole as shown on the Plans. No extra payment shall be made for manhole excavation.
- 38.10 The concrete base shall have a minimum thickness of 6 inches measured from bottom of slab to bottom of all pipe connecting to the manhole and shall be poured on undisturbed earth. The base shall be poured so that the top of the concrete base outside the barrel is one-half of the pipe diameter of all pipes entering the manhole. The base shall extend 1 foot outside of the finished manhole barrel. Prior to pouring the base, any water in the excavation shall be removed, and the base poured in the dry. Reinforcement steel consisting of #4 rebar (1/4 inch diameter) shall be placed at mid depth in the base at 12inch centers in two directions.
- 38.11 Excavation for cast-in-place manhole footings shall be limited to the area to be filled with concrete.
- 38.12 Contractor shall support all pipes from the manholes back to solid bedding by filling under the pipe and up to one-half the outside diameter, with concrete. All pipes entering a manhole shall extend to the interior surface of the manhole barrel.
- 38.13 The invert of the manhole shall be benched and hand shaped with concrete as specified. The base and barrel of the manhole shall be cleaned thoroughly prior to placement of the benched invert. Benches shall be shaped and sloped ½ inch/foot from the manhole barrel wall down to ½ of the inside pipe diameter. The entire diameter of each pipe entering the manhole barrel shall be cut smooth with the inside surface of the manhole barrel and the invert shaped smooth from all inlet pipes to the outlet pipe. The flow channels in the invert shall be contoured

## Cabot WaterWorks Standard Construction Specifications

to the inside diameter of all pipes entering and exiting the manhole. Flow channels shall have the same slope between inlet and outlet as the connected pipe.

- 38.14 Manhole Barrels. The minimum thickness of manhole barrels shall be 6 inches. The barrel may be poured monolithically with the base, or the barrel forms may be set as soon as the concrete base has cured enough to support the forms. Barrel forms shall be set plumb and properly braced prior to pouring concrete. Vibration equipment shall be used while pouring the barrel walls to eliminate voids in the concrete barrel wall.
- 38.15 All manholes shall be 4 feet inside diameter, except where larger diameter manholes are called for on the Plans.
- 38.16 Prior to setting the manhole barrel forms in place, any water that may have accumulated in the excavated area shall be pumped out and the concrete base thoroughly cleaned, if required, of dirt and debris. All concrete shall be poured in the dry.
- 38.17 Construction joints shall be provided with a keyway as shown on the detail and shall have a 2 inch layer of grout applied to the previous pour immediately before the next wall lift is poured.
- 38.18 The cast-in-place forms may be removed after the initial set of the concrete so that holes may be cut in the manhole barrel for the installation of pipes which are to enter the manhole at points other than adjacent to the manhole base. After these pipes have been put in place, the barrel shall be repaired using a non-shrink grout mixture conforming to ASTM C1107.
- 38.19 After removal of the forms all honeycombing, imperfections and depressions found in the barrel wall shall be immediately filled with mortar and rubbed with a wooden float or steel trowel to give a uniform appearance. Spurs shall be removed from the concrete surface.
- i. Barrels exposed above grade shall be hand rubbed as required. Wet and fill all voids using mortar with the same sand-cement ratio as original concrete. Blend cement to match concrete color. Strike off all excess mortar flush with the surface using a burlap or canvas cloth with a circular motion. Remove all rough spots and rub with cloth to leave a surface of uniform texture and appearance. Finish shall result in a coating of mortar that will fill all voids and air holes, leaving a smooth surface.
- 38.20 Manhole steps are not required.
- 38.21 Top of manholes shall be constructed 1 foot above finished grade; 1 foot above flood elevation and/or 1 foot above peak storm flow elevation when located adjacent to open storm drainage channels. Exception: Top of manholes located at high elevations that will not collect storm water runoff, may be constructed 2" above finished grade.
- a). Cabot Waterworks may consider bolted water tight manhole lids as an option.
  - b). Finish grade shall slope away from manholes, (including any paved surface)
  - c). Top of manholes located in paved surfaces shall be set at the final grade of the paved surface.
- 38.22 Manholes shall be installed at the end of each line and spaced at distances no greater than 600 feet.
- 38.23 Drop manholes, unless otherwise shown on the Plans, shall be constructed at all manholes where the difference in invert elevation between incoming and outgoing sewer is 2 feet or more. Drop manholes shall be constructed of the same material and dimensions as are standard manholes, the only difference being in the inlet arrangements as shown on the standard detail.
- 38.24 Curing compounds or covers may be used at the option of the Contractor. However, it will be the responsibility of the Contractor to protect the concrete to prevent cracking during the curing process and to protect the manhole during freezing temperatures. The Engineer shall, at his discretion, prohibit pouring concrete during periods of extreme cold or inclement weather.
- 38.25 Manholes shall be backfilled only after 48 hours of concrete curing time. Extra care shall be taken to compact all backfill evenly around the perimeter of the manhole to the top of the highest pipe entering the manhole. Backfill material shall conform to pipe bedding as specified elsewhere in these Specifications.
- 38.26 All manholes constructed shall be watertight and show no visible evidence of infiltration.
- 38.27 Manholes shall pass a vacuum test in accordance with ASTM Designation C1244

## Cabot WaterWorks Standard Construction Specifications

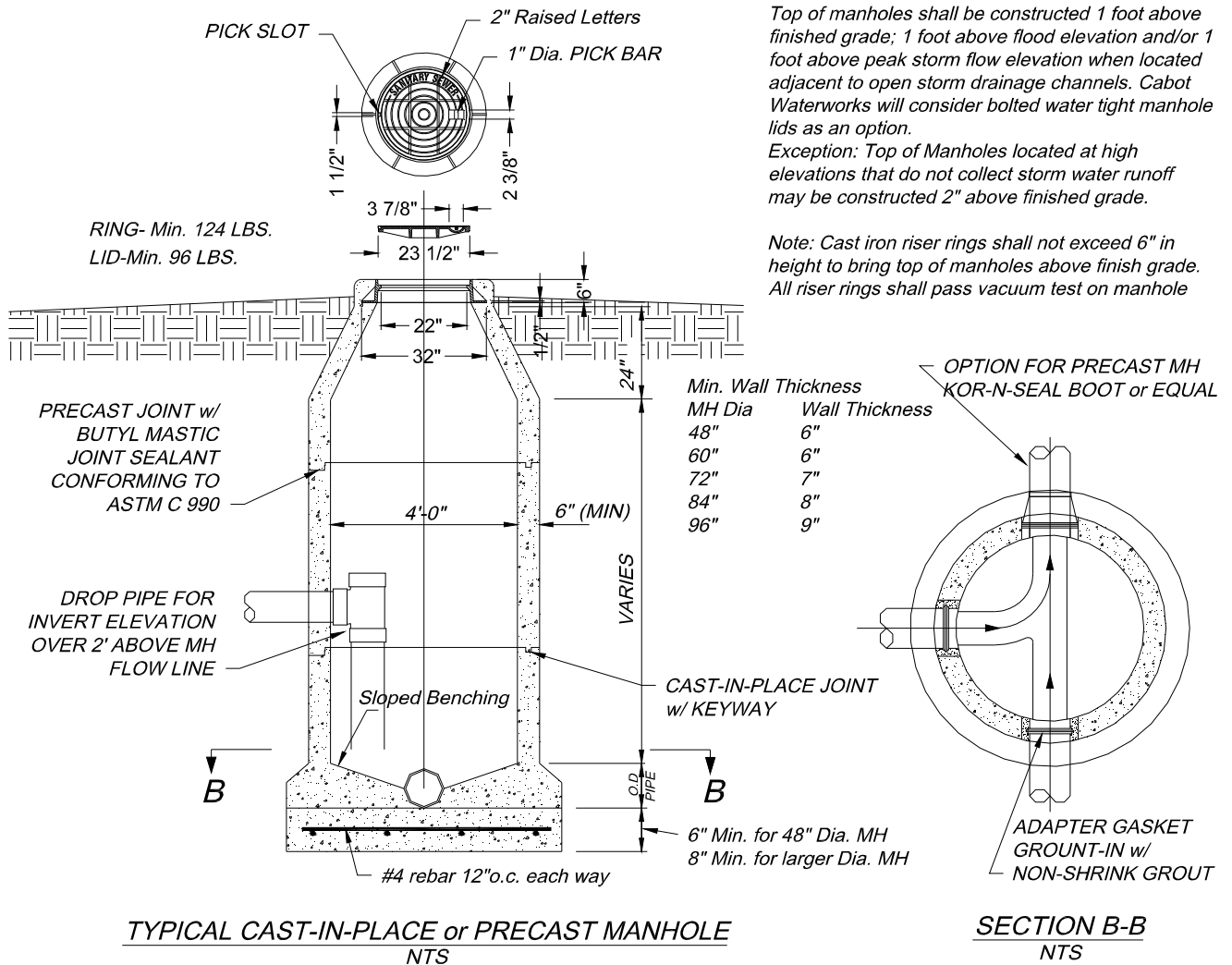
38.28 Vacuum Test. All incoming and outgoing sewer lines shall be plugged and vacuum drawn on the manhole. The Contractor shall furnish a suitable apparatus, such as manufactured by P. A. Glazier, Inc., made for such purpose. The test shall be performed by drawing a vacuum of 10 inches of mercury (Hg) (equivalent to 5 PSI vacuum) and measuring the time for a drop to no more than 9 inches Hg, (½ PSI vacuum drop). The time measured shall not be less than indicated in the table below for a 1 inch Hg, (½ PSI) drop in vacuum.

<u>Depth of MH</u> (feet)	<u>Time in seconds for 1" Hg (1/2 PSI) loss of vacuum</u>		
	<u>48" Diameter</u>	<u>72" Diameter</u>	<u>96" Diameter</u>
4'	10	16	23
6'	15	25	34
8'	20	33	45
10'	25	41	56
12'	30	49	67
14'	35	57	78
16'	40	67	89
18'	45	73	100
20'	50	81	111

38.29 For all manholes with leaks in excess of the above, the CONTRACTOR shall, at his own expense, perform whatever work and/or replace whatever material that is required in order to remedy the defect and stop the leaks. All corrective work shall be approved by CWW.

38.30 Manholes which fail the test shall be retested after remedial measures are completed.

## Cabot WaterWorks Standard Construction Specifications



Top of manholes shall be constructed 1 foot above finished grade; 1 foot above flood elevation and/or 1 foot above peak storm flow elevation when located adjacent to open storm drainage channels. Cabot Waterworks will consider bolted water tight manhole lids as an option.  
Exception: Top of Manholes located at high elevations that do not collect storm water runoff may be constructed 2" above finished grade.

Note: Cast iron riser rings shall not exceed 6" in height to bring top of manholes above finish grade. All riser rings shall pass vacuum test on manhole

Typical Manhole Detail – Cast-in-Place or Pre-Cast

### 39. WASTEWATER GRAVITY SEWER PIPELINE

- 39.1 All requirements for Tracer Wire and Marking Tape of Section 11 of these specifications shall apply to wastewater pipeline installation.
- 39.2 All joints shall be of the bell and spigot type and conform to ASTM D3212 and/or Uni-Bell UNI-B-1. Gaskets shall be in accordance with ASTM F477. All bells shall be formed integrally with the pipe and shall contain a factory installed elastomeric gasket which is positively retained. Wall thickness of this bell at any point shall not be less than the required minimum for the pipe barrel. No solvent cement joints will be permitted in field construction except as specifically authorized by the Engineer.
- 39.3 Gasket lubricants shall be supplied as per the pipe manufacturer recommendations. Contractor shall provide documentation that gasket lubricants are approved by the manufacturer for the pipe and gaskets used on the project. Lubricants shall be applied as per pipe manufacturer's recommendation.
- 39.4 Construction Sequence. Construction of sewers shall begin at the low point of the line and continue in orderly succession throughout the project. Any deviation from this procedure shall be made only with the specific approval of the Engineer. Pipe shall be laid with bell ends facing the direction of laying upgrade.

## Cabot WaterWorks Standard Construction Specifications

- i. Gravity sewer pipe shall be installed in straight alignment between manholes at the grade indicated on plans.
- 39.5 Connections to new manholes shall conform to Section for "Sewer Manholes".
- 39.6 Existing Manholes. Connections to existing manholes or inlets, where no plugged stubs exist, shall be made by cutting a hole in the wall of the existing structure, inserting the PVC pipe with a manhole adapter around the pipe into the hole, filling the annular space around the pipe with non-shrink grout, and troweling the inside and outside surface to a smooth finish. The bottom of the manhole shall be shaped to fit the invert of the sewer pipe.
- 39.7 Contractor shall support all pipes from manholes back to solid bedding by filling under the pipe and up to one-half the outside diameter, with concrete.
- i. Backfill shall be placed as specified elsewhere in these Specifications.
- 39.8 Contractor shall conduct camera inspection of all new sanitary sewer lines with a closed circuit television system. The sewer line will be flushed with clean water prior to commencing the T.V. inspection. Trapped or standing water in excess of ¼" will be considered defective work and shall be corrected by the Contractor prior to acceptance.
- 39.9 Mandrel Testing. Gravity PVC sewer mains shall be tested for deflection in accordance with ASTM D2321.
- i. Mandrel test shall not be performed for at least 30 days after reaching final trench backfill grade, provided, in the opinion of the Engineer, that sufficient water densification or rainfall has occurred to thoroughly settle the soil throughout the entire trench depth. If this has not been achieved prior to the project completion date, then the Mandrel size shall be increased to measure one-third less of a deflection allowance.
  - ii. Flexible sewer pipes shall be Mandrelled with a rigid device sized to pass 5 percent or less deflection (or deformation) of the pipe.
  - iii. The Mandrel (go/no-go) device shall be cylindrical in shape and constructed with a minimum of 9 evenly spaced arms or prongs. Mandrels with fewer arms will be rejected as not sufficiently accurate. The contact length of the Mandrel's arms shall equal or exceed the nominal diameter of the sewer to be inspected. Critical Mandrel dimensions shall carry a tolerance of plus or minus 0.01 inch.
  - iv. Inside pipe dimensions specified in the applicable ASTM pipe standards, shall be used to compute the Mandrel dimensions.
  - v. The Mandrel shall be hand-pulled by the Contractor through all sewer lines. Any sections of sewer not passing the Mandrel shall be uncovered and the Contractor shall re-bed, re-round or replace the sewer pipe to the satisfaction of the Engineer. These repaired sections shall be retested.
  - vi. Drawings of the Mandrel with complete dimensioning shall be furnished by the Contractor to the Engineer for his approval for each diameter and specification of pipe.
- 39.10 Alignment Tests - Each section of sewer between manholes shall be lamped by the Contractor in the presence of the Engineer to determine whether any displacement of the pipe has occurred. The Contractor shall repair any misalignment, displaced pipe, or other conditions which deviate from those specified.
- 39.11 Low Pressure Air Testing for Sewer Lines.
- i. Contractor shall conduct low pressure air tests on all gravity sewer lines in accordance with ASTM F1417. The equipment required for the test shall include a regulator to avoid over-pressurizing and possibly damaging an otherwise acceptable line. The equipment used shall be Air-Loc system as manufactured by Cherne Industrial, Inc., Hopkins, Minnesota, or equal.
  - ii. The low pressure air test shall be conducted by plugging each opening in the reach of pipe to be tested. All plugs shall be braced against slippage due to internal pressure, and no one shall be allowed in the manhole during the testing procedure. One of the plugs provided must have an inlet tap or other provision for connecting an air hose. After connecting the air control equipment to the air hose, monitor the air pressure so that the internal pressure does not exceed 5.0 psig. After reaching 4.0 psig, the air supply shall be throttled to maintain between 4.0 and 3.5 psig for at least 2 minutes in order to allow equilibrium between air temperature and the pipe walls. If plugs are found to leak, the Contractor shall bleed off the air, tighten the plugs, and again begin supplying air. After the temperature has stabilized, the pressure is allowed to decrease to 3.5 psig. At

## Cabot WaterWorks Standard Construction Specifications

3.5 psig, the Contractor shall begin timing to determine the time required for the pressure to drop to 3.0 psig. The pipe shall be presumed free of defects if the time, in seconds, for the air pressure to decrease from 3.5 psig to 3.0 psig is greater than that shown in the table shown on the following page.

- iii. Any test section less than 100 feet in length shall be tested for the time set out for 100 feet.
- iv. If by use of the above procedure, defective section(s) of pipeline are found, the pipeline segment shall be tested at 20 foot intervals to determine the exact location of the defect(s). Repairs shall be made in defective section(s) and the entire line segment shall then be retested.

### SPECIFICATION TIME REQUIRED FOR 0.5 PSIG PRESSURE DROP FOR SIZE AND LENGTH OF PIPE INDICATED FOR Q = 0.0015

Pipe Dia. (in.)	Min. Time (min:sec)	Length for Min. Time (ft)	Time for Longer Length (sec)	Specification Time for Length (L) Shown (hr:min:sec)										
				100	150 ft.	200 ft.	250 ft.	300 ft.	350 ft.	400 ft.	450 ft.	500 ft.	550 ft.	600 ft.
8	3:47	298	.760L	0:03:47	0:03:47	0:03:47	0:03:47	0:03:48	0:04:26	0:05:04	0:05:42	0:06:20	0:06:58	0:07:36
10	4:43	239	1.187L	0:04:43	0:04:43	0:04:43	0:04:57	0:05:56	0:06:55	0:07:55	0:08:54	0:09:54	0:10:53	0:11:52
12	5:40	199	1.709L	0:05:40	0:05:40	0:05:42	0:07:07	0:08:33	0:09:58	0:11:24	0:12:49	0:14:15	0:15:40	0:17:05
15	7:05	159	2.671L	0:07:05	0:07:05	0:08:54	0:11:08	0:13:21	0:15:35	0:17:48	0:20:02	0:22:15	0:24:29	0:26:43
18	8:30	133	3.846L	0:08:30	0:09:37	0:12:49	0:16:01	0:19:14	0:22:26	0:25:38	0:28:51	0:32:03	0:35:15	0:38:28
21	9:55	114	5.235L	0:09:55	0:13:05	0:17:27	0:21:49	0:26:11	0:30:32	0:34:54	0:39:16	0:43:37	0:47:59	0:52:21
24	11:20	99	6.837L	0:11:24	0:17:06	0:22:47	0:28:29	0:34:11	0:39:53	0:45:35	0:51:17	0:56:59	1:02:40	1:08:22
27	12:45	88	8.653L	0:14:25	0:21:38	0:28:51	0:36:03	0:43:16	0:50:29	0:57:41	1:04:54	1:12:07	1:19:19	1:26:32
30	14:10	80	10.683L	0:17:48	0:26:42	0:35:37	0:44:31	0:53:25	1:02:19	1:11:13	1:20:07	1:29:02	1:37:56	1:46:50
33	15:35	72	12.926L	0:21:33	0:32:19	0:43:05	0:53:52	1:04:38	1:15:24	1:26:10	1:36:57	1:47:43	1:58:29	2:09:16
36	17:00	66	15.384L	0:25:38	0:38:28	0:51:17	1:04:06	1:16:55	1:29:44	1:42:34	1:55:23	2:08:12	2:21:01	2:33:50

39.12 ACCEPTANCE INSPECTION BY CCTV: Before acceptance of gravity sewer pipelines by CWW, gravity sewer line segments shall be inspected for final acceptance by CCTV.

- i. A Cabot Waterworks representative shall be present during the CCTV inspection.
- ii. Sewer Mains shall be flushed and all debris removed prior to CCTV inspection.
- iii. The television camera used for the inspection should be designed and constructed specifically for such inspection, producing a highly legible picture. The camera shall be operative in conditions 100% humidity and/or under water. The lighting and camera quality shall be suitable to allow a clear, in focus picture of a minimum of 6 linear feet of the entire inside periphery of the sewer pipe.
- iv. Picture quality and definition shall be to the satisfaction of Cabot Waterworks.
- v. The section of gravity sewer line being inspected shall be isolated from the remainder of the sewer line as necessary. The Contractor shall make all provisions for pumping or bypassing the flow around the manhole section as required and the cost shall be incidental to the CCTV inspection.
- vi. The camera shall be moved through the sewer line in either direction at a uniform slow rate not to exceed 60 feet per minute. Under no circumstances shall the camera be tethered to a hydraulically propelled or high velocity jet cleaning device while the cleaning device is on.
- vii. The camera shall stop at each service connection and provide a view up the service line.
- viii. Contractor shall not be allowed to float the camera.
- ix. The Contractor shall furnish DVD's of the lines televised to CWW for review and comments, which may require up to 30 calendar days from the date submitted. Each DVD shall be labeled with the following information:
  - a). Project Job Number
  - b). Manhole to Manhole Designation

## Cabot WaterWorks Standard Construction Specifications

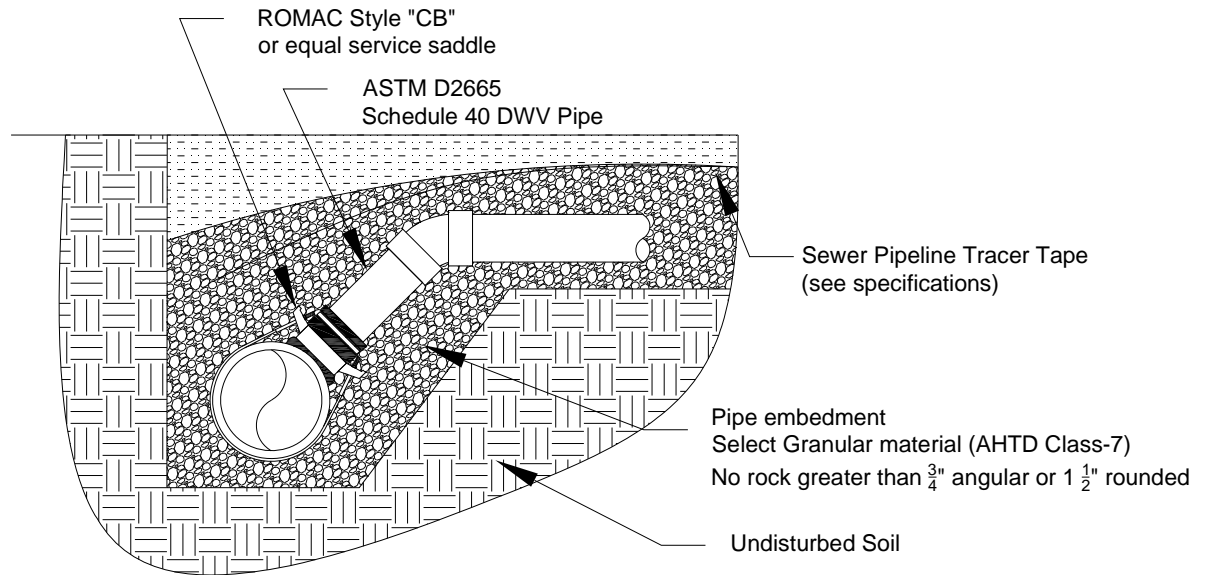
- c). Date Televised
- d). Street or Other Location
- x. The DVD recordings shall log the following information:
  - a). Project Job Number (or Subdivision Name)
  - b). Name of Contractor
  - c). Date Televised and Continuous Distance Log (measured in feet)
  - d). Street or Other Location
  - e). Upstream Manhole Designation
  - f). Downstream Manhole Designation
  - g). Pipe Material and Diameter
  - h). Direction of Televising (Downstream or Upstream)
- xi. DVD's shall become the property of the CWW and will be retained by the CWW.
- xii. If the DVD's are of such poor quality that CWW is unable to evaluate the condition of the sewer line or to locate service connections, the Contractor shall be required to re-televising and provide a good quality DVD of the sewer line at no additional cost to CWW.
- xiii. All sections of sewer pipe that fail to drain after flushing will be considered unacceptable. The cost of such repairs shall be the responsibility of the Contractor.
- xiv. All sections of sewer lines considered unacceptable shall be repaired using new materials. The cost of such repairs shall be the responsibility of the Contractor.
- xv. Cabot Waterworks shall make no payments for repair of unsatisfactory or defective work.

### **40. Wastewater Service Connections**

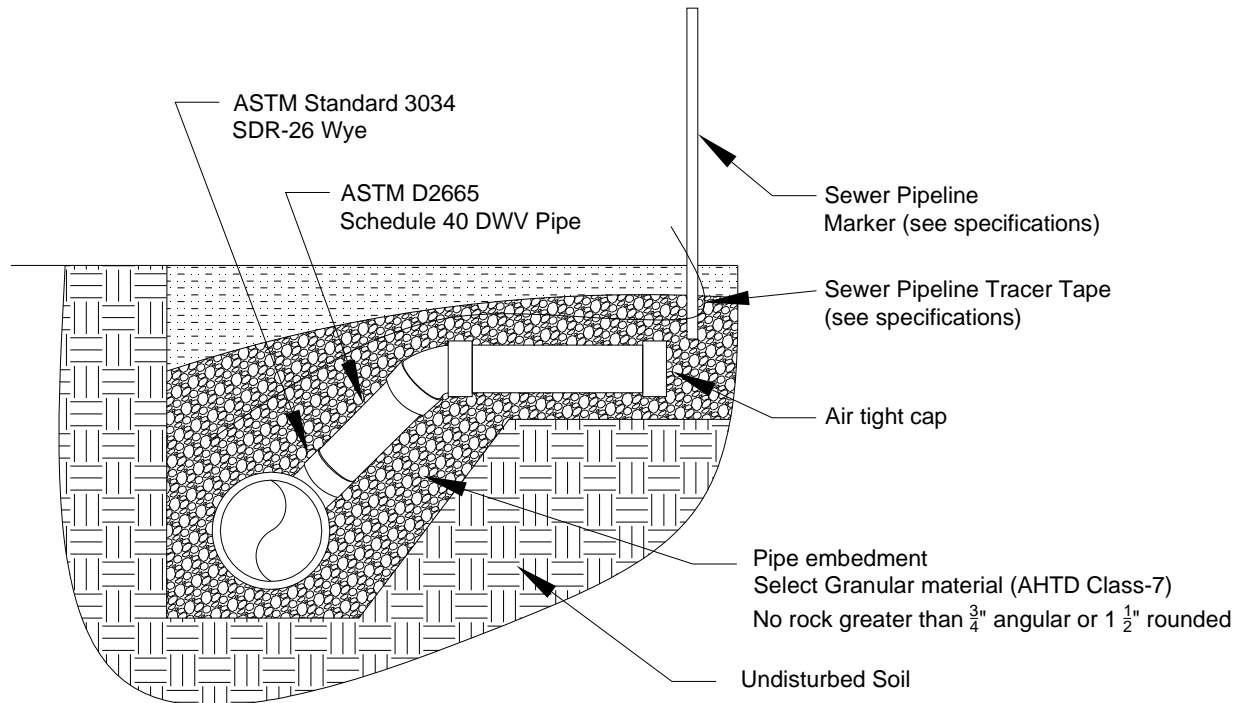
- 40.1 **All requirements for Tracer Wire and Marking Tape of Section 11 of these specifications shall apply to wastewater service installation.**
- 40.2 Service pipe material shall conform to ASTM D 2665, Schedule 40 DWV. The minimum diameter for service pipe shall be 4".
- 40.3 Building sewer service lines connecting to Cabot WaterWorks sewer system shall be sized and installed in accordance with the Arkansas Plumbing Code.
- 40.4 Cabot WaterWorks shall be notified prior to any connection to the Cabot WaterWorks sewer system. All building sewer service lines and connections to Cabot WaterWorks sewer system shall be inspected by CWW prior to covering or backfilling the connection.
- 40.5 Wastewater service connections shall be made at manholes where possible. Service connections at manholes shall conform to all requirements of pipeline connections to manholes specified in Sections 38 and 39 above.
- a). Service connections to manholes shall be made at an elevation above the manhole invert of at least 2/3 the main line pipe diameter though not more than 24".
- 40.6 Contractor shall install service wyes and stub-out sewer service lines to every building lot in new subdivision construction. Service wyes shall conform to ASTM D3034, SDR-26. Service stub-outs shall be capped with water tight caps. No plugs will be permitted. Service stub-outs shall be subjected to pressure test when the sewer main is tested.
- a). The end of Service line stub-outs shall be marked with a green steel fence post with a 2 1/2" PVC pipe slipped over the post with "Sewer Pipeline" stenciled on the PVC pipe in green color. Alternatively a 66" tall, green colored fiberglass marker labeled "Sewer Pipeline" maybe installed at the end of the service stub-outs. The markers shall not be disturbed during construction on the lot.
- 40.7 Service connections to existing sewer mains shall be performed using a service saddle. Service saddles shall be ROMAC Industries Style "CB" or equal. Service saddles shall conform with the following requirements:
- a). The saddle body cast from ductile iron, meeting ASTM A536, Grade 65-45-12 with epoxy coating.
  - b). Gasket from virgin Styrene Butadiene Rubber (SBR) in accordance with ASTM D 2000 MBA 710.
  - c). A pipe stop molded into the inside wall of the gasket and rated for 1000 lbs. of force along the branch.
  - d). A Stainless Steel strap, type 304 (18-8), 3 1/2 inches wide

## Cabot WaterWorks Standard Construction Specifications

- e). Bolts and nuts Type 304 (18-8) Stainless Steel 1/2" National Coarse roll thread.
- f). Rated for a 7 psi air test when properly installed.



### Typical Sewer Service Connection to Existing Sewer Main



### Typical Sewer Service Connection to New Sewer Main

## Cabot WaterWorks Standard Construction Specifications

40.8 Service Saddles shall be installed in accordance with manufacturer's recommendations. As directed by the Engineer, Contractor shall pressure test service connections in accordance with the **Section 39, WASTEWATER GRAVITY SEWER PIPELINE** of these specifications.

### 41. Grease Traps and Grease Interceptors

41.1 Grease traps and grease interceptors shall be installed as required by the Arkansas State Plumbing Code and Cabot WaterWorks.

41.2 Traps shall be installed at a minimum distance of 10 ft. from sinks and dishwashers to allow for adequate cooling of the wastewater. Water temperatures should be less than 140 degrees prior to entering grease trap. All grease bearing waste streams should be routed through a grease trap or interceptor, including: three-compartment sinks, pot/pan sinks, hand-washing sinks, dishwashers, mop sinks and floor drains.

41.3 Grease traps shall have two compartments and constructed of pre-cast concrete. Very large units may be cast-in-place. Manholes to finished grade shall be provided. Cabot WaterWorks requires approval of the design of grease traps before the grease trap is purchased or installed.

41.4 The minimum capacities of grease traps shall be from the EPA Design method below for restaurants and commercial kitchens, though not less than 1000 gallons.

$$(M) \times (GL) \times (ST) \times (2.5) = \text{Size of Grease Trap in gallons.}$$

Where:

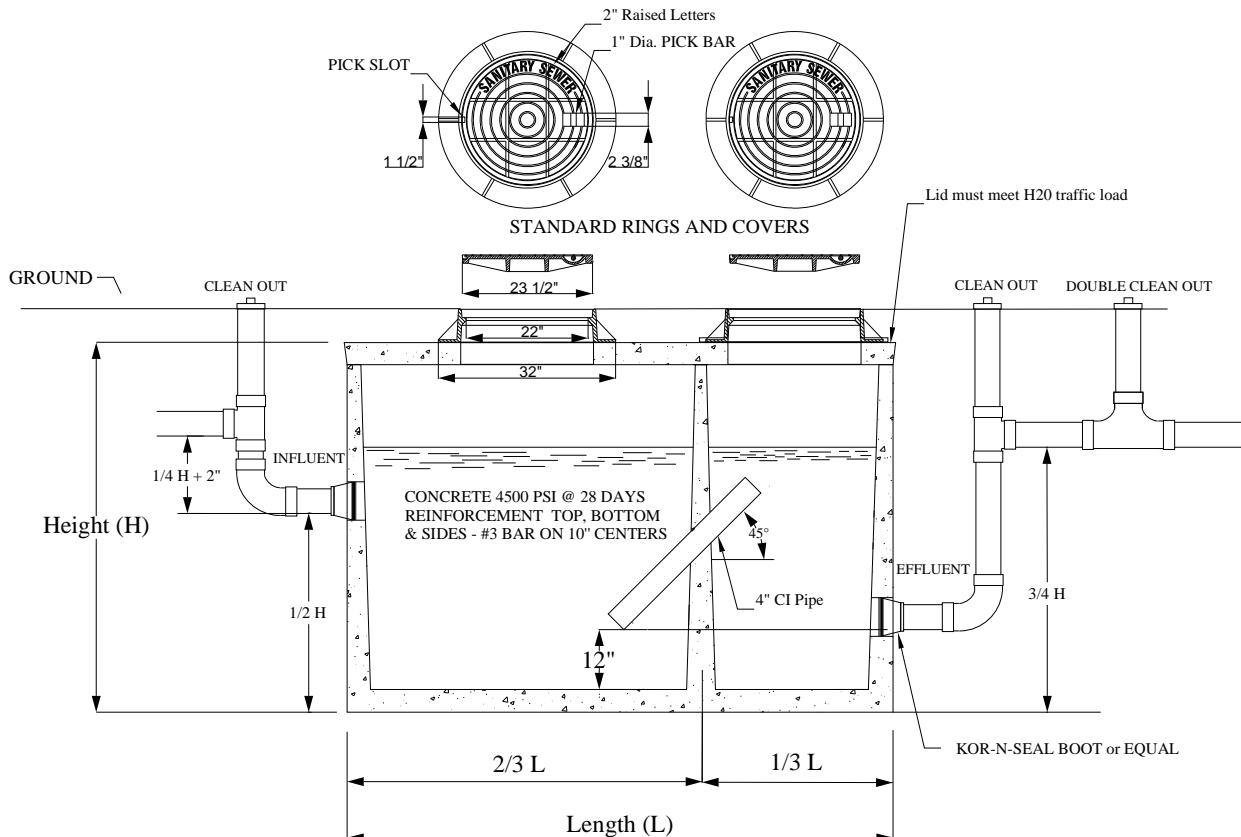
M = Meals per day

GL = Gallons of wastewater per meal, normally 4.5 gal

ST = Storage capacity factor -- minimum of 1.7

**For a restaurant or kitchen preparing 100 meals per day, the minimum size of grease trap will be as follows, (100) x (4.5) x (1.7) x (2.5) = 1900 gals.**

41.5 Grease trap construction shall conform to the typical detail below.



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### Typical Grease Trap Detail

41.6 Grease traps shall be placed on undisturbed earth. Prior to installing, any water in the excavation shall be removed, and the tank set in the dry.

#### **42. Oil Separators and Sand Interceptors**

42.1 Oil separators and sand interceptors shall be installed as required by the Arkansas State Plumbing Code.

42.2 Plans shall be submitted to Cabot WaterWorks and approved prior to construction, for facilities requiring Oil separators and Sand interceptors.

#### **43. Wastewater Lift Stations**

43.1 Wastewater lift stations shall be duplex non clog submersible sewage pump stations and shall include two (2) pumps, stainless steel lifting chains, stainless steel guide rails and brackets, cast in place aluminum access hatch, recessed hoist socket, duplex control panel, stainless steel float switch bracket and mercury float switches and all other materials required for installation into a concrete wet well and valve vault. Lift stations shall be constructed in accordance with the Typical Lift Station Detail below.

43.2 Wet well and Valve Vault shall be constructed with pre-cast concrete manhole sections conforming to ASTM C478. Pre-cast manhole joints shall be sealed using a butyl mastic sealant conforming to ASTM C990. Pipe connections to wet well shall be made with a manhole adapter gasket conforming to ASTM C923 and designed to connect PVC sewer pipe to a concrete manhole wall. Manhole Adapter Gaskets shall be equal to Romac Style LCT or FERNCO Inc. and grouted into manhole openings.

- i. The concrete base shall have a minimum thickness of 12 inches and shall be poured on undisturbed earth. The base shall extend 1 foot outside of the finished manhole barrel. Prior to pouring the base, any water in the excavation shall be removed, and the base poured in the dry. Reinforcement steel consisting of #4 rebar (1/4 inch diameter) shall be placed at mid depth in the base at 12inch centers in two directions.

43.3 Wet well and valve vault cover shall be constructed with an aluminum cast in place hatch covers of adequate size to permit pump removal as indicated on the Typical Lift Station Detail below. Access hatch shall have stainless steel safety chains around openings.

43.4 Hoist Socket – A stainless steel embedded style hoist socket with a minimum weight rating of 1330 lbs. shall be provided for installation by contractor. The socket shall be a Model D3E as manufactured by Halliday Products.

43.5 Pump manufacturer shall be FLYGT or equal and classified as non-clog submersible. All openings in pump shall be large enough to pass a 3” diameter sphere. Discharge shall be 4 inch diameter minimum, standard flange. Pump manufacturer must have an authorized service center within 50 miles of Cabot.

43.6 Pump operating conditions, design calculations shall be submitted by a Professional Engineer to Cabot Waterworks for approval of pump operating conditions prior to pump procurement. Pumps will not be accepted by Cabot WaterWorks until pump sizing calculations have been approved by Cabot WaterWorks.

43.7 Motor - Pump motor shall be sealed submersible, NEMA B type, 60 hertz, 480 volt, three (3) phase (where 3 phase power is available. Motor shall have a service factor of 1.15 and have a voltage tolerance of +/- 10%.

- a). Stator winding shall be of the open type with class H insulation good for 356°F maximum operating temperature. Winding housing shall be filled with clean dielectric oil that lubricates bearings and seals and transfers heat from windings and rotor to outer shell.
- b). Motor shall have two (2) heavy duty ball bearings to support pump shaft and take radial and thrust loads and sleeve guide bushing directly above the lower seal to take the radial load and act as flame path for seal chamber. Ball bearings shall be designed for 50,000 hours B 10 life. Stator shall be heat shrunk into motor housing.
- c). A heat sensor thermostat shall be attached to and imbedded in the winding and be connected in series with the motor starter coil to stop motor if temperature of winding is more than 248°F. Thermostat shall reset automatically when the motor cools to a safe operating temperature. Three (3) heat sensors shall be used and common pump motor shaft shall be 416 stainless steel.
- d). Seals Motor shall be protected by two (2) mechanical seals mounted in tandem with a seal chamber between the seals. Seal chamber shall be oil filled to lubricate seal face and to transmit heat from the

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shaft to outer shell. Upper seal face shall be carbon and ceramic and lapped to a flatness of one (1) light band. Lower seal shall be tungsten carbide. A double electrode shall be mounted in the seal chamber to detect any water entering the chamber through the lower seal. Water in the chamber shall cause a red light to illuminate in the control panel. This signal shall not stop the motor but shall act as a warning only, indicating service is required.

- e). Impeller The impeller shall be ductile iron and of the 2 vane non-clog enclosed type. Pump out vanes shall be used in front and back chamber. Impeller shall be dynamically balanced. Impeller shall be driven by stainless steel key and impeller held in position with lock screw and washer. Impeller and motor shall be capable of top lift out so the assembly can be removed from bottom volute without removing impeller.
  - f). Pump Case The volute case shall be cast iron and have a flanged center line discharge. A bronze wear ring shall be pressed into case for guiding impeller neck and to prevent corrosion freeze up. Wear ring to be held from rotating by locking with stainless steel set screw in end of ring.
  - g). Pump and Motor Casting shall be high tensile cast iron and shall be treated with phosphate and chromate rinse. All fasteners shall be 302 stainless steel.
  - h). Bearing End Cap Upper motor bearing cap shall be a separate casting.
  - i). Power Cables Power cord and control cord shall be triple sealed. The power and control conductor shall be single strand sealed with epoxy compound and then clamped in place with rubber seal bushing to seal outer jacket against leakage and to provide for strain pull. A third sealing area shall be provided by a terminal board to separate the cable entry chamber from the motor chamber. Cords shall withstand a pull of 300 pounds. Insulation of power and control cords shall be type SOOW. Both control and power cords shall have a green carrier ground conductor that attaches to motor frame.
- 43.8 Base Disconnect System Each pump shall be equipped with necessary components to allow pumps to be removed from wet well without the need to enter wet well. Components shall be provided by the pump manufacturer as follows.
- a). A ductile iron discharge base, cast iron pump, attaching and sealing plate, cast iron pump guide plate and cast iron elbow. All exposed nuts, bolts and fasteners shall be 300 series stainless steel. No fabricated steel parts shall be used.
  - b). Discharge elbow shall be 4" x 4" and shall bolt onto discharge base and have standard 125 lb. flanges.
  - c). Pumps shall have a sealing plate and guide plate bolted to the pump. Pumps shall be automatically connected and sealed to the base when slid down the guide rails. The open face of the sealing plate shall have a dove-tailed groove machined into the face to hold a sealing O-ring. The O-ring shall provide a leak-proof seal at all pressures.
  - d). Guide Rails shall have two (2) schedule 40 stainless steel rail pipes used to guide the pump from the surface to the discharge base. The weight of the pump shall bear solely on the discharge base and not on the guide rails. Systems which require the pump to be supported by legs are not acceptable. The guide rails shall be firmly attached to the access hatch frame with stainless steel upper brackets. Systems deeper than 20' shall use a stainless steel intermediate guide for each 20' of wet well.
  - e). A stainless steel lifting chain shall be supplied for each pump and shall be of sufficient length and include an adequate number of lifting rings for easy removal.
- 43.9 Wet Well Piping - All pipe and elbows above the pump discharge elbow within the wet well shall be type 304 Stainless Steel pipe schedule 40. All Bolts used on discharge base and piping inside the wet well shall be stainless steel. All piping shall be restrained joint connections, (flanged connections).
- 43.10 Valve vault piping shall conform to the Lift Station Detail below.
- a). Flanged Ductile Iron pipe, cement lined or epoxy coated conforming to AWWA C115, pressure rating 250psi.
  - b). Ductile Iron Pipe Fittings in valve vault shall conform to AWWA C110/ ANSI A21.10, pressure rating 250psi. Fittings shall have an epoxy coating conforming to AWWA C116/ANSI A21.16.
  - c). Gate Valves - Isolation gate valves shall be provided conforming to AWWA standard C-509.

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- d). Check Valves - Check valves shall conform to AWWA C508 for Swing-Check Valves w/ external lever.

43.11 Control Panel – The control panel shall be equipped with the following components and operation:

- a). NEMA 12/3R Enclosure, Enamel painted steel, w/ Aluminum inner door. Control panel shall comply with UL508/NEC Article 409 – SCCR.
- b). All switches, lights and overload resets shall be mounted through the inner door.
- c). Incoming Power Terminals, w/neutral and ground
- d). NEMA Heavy Duty Circuit Breakers
- e). NEMA Full Voltage Motor Starters, **starting capacitors are not permitted**
- f). High Level Alarm with flashing alarm light and horn w/silence Button
- g). 50W Heater with Thermostat, UL 508 serialized
- h). As-Built schematic

43.12 Each pump shall have separate operating controls consisting of;

- a). Separate three phase power monitors
- b). Separate Control Circuit Breakers
- c). Separate Control Power Transformers
- d). NEMA 4X, H-O-A switches w/ Run Lights
- e). Elapsed Hour Meters
- f). Pump run lights, pump seal fail lights, pump high temp lights.
- g). Each pump shall be controlled by two (2)-120 volt ac, normally open, mercury float switches. The “Pump start” float shall activate a latching relay to start the pump. The “Pump off” float shall hold the relay closed until the water level drops below the pump off float.
- h). A mechanical switch connected to both sets of floats shall be used to manually alternate lead and lag pumps.

43.13 Equipment Start Up - The pump equipment supplier shall provide a trained service technician to certify proper equipment installation and operation.

43.14 **Warranty - The pump equipment supplier shall provide CWW a written five (5) year warranty from date of equipment start up. Any defective portions of pump equipment and control panel, whether materials or workmanship, shall be replaced by pump equipment supplier at no cost to CWW during the term of the warranty.**

43.15 Lift Stations shall be equipped as follows.

- a). Onsite Automatic auxiliary generator with automatic transfer switch will be required on lift stations above 250 GPM. Auxiliary generators shall be sized to operate all pumps and equipment installed at the station. Fuel tanks for generators shall be sized for 24hour continuous operation of the lift station. For stations of less than 250 GPM a portable auxiliary generator connection will be supplied by developer to meet Cabot WaterWorks standards.
- b). All materials and equipment exposed to sewer gas shall be corrosion resistant.
- c). Discharge piping outside of wet well shall have isolation valves and check valves installed in a concrete vault with adequate space to permit maintenance.
- d). Emergency pump connection shall be provided for bypass of pump station and fitted with male Camlock adapter with cap. Bypass shall be located in valve vault downstream of pump isolation valves and extend 2 feet above valve vault.
- e). Flow metering equipment installed that provides total and current flow rate.

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- f). Supervisory Control and Data Acquisition (SCADA) equipment shall be installed that monitors and alarms the following:
- Pump 1 or 2 – Run Fail, (starter does not pull in).
  - Power Failure at lift station.
  - Communication Failure at lift station.
  - High Water level alarm at station.
  - Phase Failure at station
  - SCADA equipment shall consist of the following components:
    - (1). 24" X 24" X 8" NEMA 4 Enclosure (Hoffman or equivalent).
    - (2). Allen Bradley Micrologix 1200 PLC.
    - (3). Microwave Data Systems MDS4700 Radio Modem.
    - (4). 12VDC 5 Amp min Power supply.
    - (5). Maxrad MYA-4505 antenna.
    - (6). Coaxial Lightning arrestor Polyphaser or equivalent.
    - (7). UPS APC-350watt or equivalent.
    - (8). 15-20' 1 ¼ antenna mast.
    - (9). LMR400 transmission line.
    - (10). The SCADA RTU shall be wired and programmed monitor and alarm the following:
    - (11). SCADA computer programming at the Wastewater Treatment Facility for the new lift station.
    - (12). Micrologix PLC programming at the Wastewater Treatment Facility for the new lift station.
    - (13). Master Polling PLC programming at the 1 Million Gallon Tank on Panther Trail for the new lift station.
- g). Paved access road.
- h). Security fencing shall be provided by chain link fence or a UV and mildew resistant vinyl fence tan color (no white). Cabot WaterWorks shall approve fencing material prior to construction. Wood fences will not be acceptable due to long term maintenance requirements.
- i). Chain-link Fence shall conform to the "Chain Link Fence Manufacturers Institute Product Manual" and the specifications below.
- (1). **Fabric:** 6' high, 9 gauge, Class 2 galvanized coating (2.0 oz. /ft<sup>2</sup>), (2" mesh) chain link. Install by securing one end and applying sufficient tension to provide a smooth uniform appearance free from sag. The fabric can be cut by untwisting a picket and attaching each span independently at all terminal posts. Use stretcher bars with tension bands or other suitable devices at 15 in. maximum intervals. The fence fabric should be installed 2" above ground level.
  - (2). **Posts and Rail Materials:** Schedule 40 Galvanized Tubing or conforming to ASTM F 1043, Group IV. High-strength steel pipe triple coated per ASTM F 1043: external coating, Type B; internal coating, Type D.
    - a. **Top Rail:** 1 5/8" OD, 2.27 lbs. per foot. Top rail 21' length, joined with 6" long couplings with .07" minimum wall thickness.
    - b. **Line Post:** 2 3/8" OD, 3.65 lbs. per foot. Line posts set 10' on center maximum spacing. Concrete footing: 8" diameter, 36" depth
    - c. **Terminal Post:** 2 7/8" OD, 5.79 lbs. per foot. Concrete footing: 10" diameter, 36" depth.

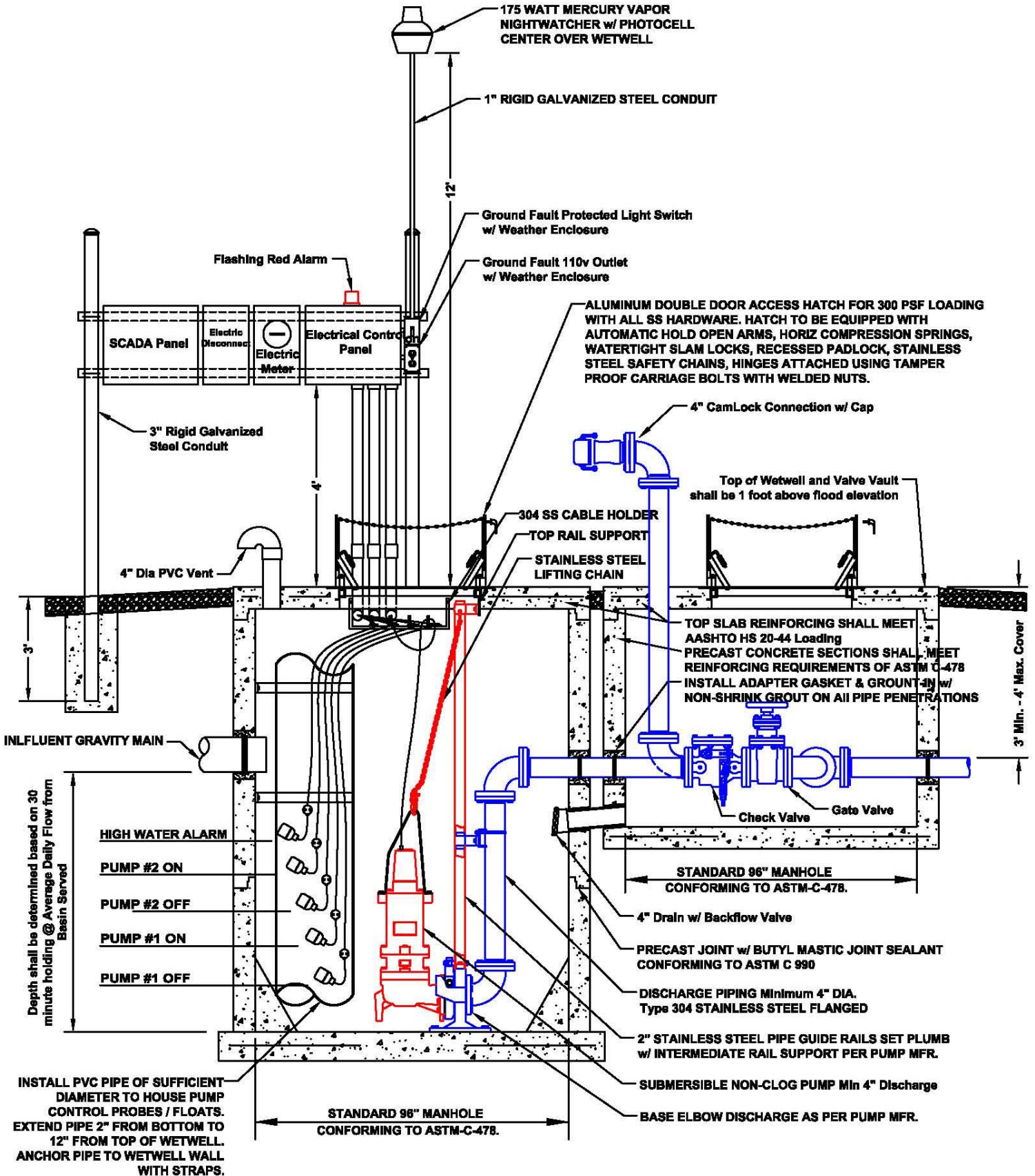
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- d. **Double swing gate Post:** 4" OD, 9.10 lbs. per foot. Concrete footing: 12" diameter, 36" depth
  - e. **Post Braces** shall be provided for each gate, corner, pull, and end post for use when top rail is omitted or with fabric 6 feet or more in height, and shall consist of a round tubular brace extending to each adjacent line post at approximately mid-height of the fabric, and a truss consisting of a rod not less than 5/16 inch nominal diameter from the line post back to the gate, corner, pull, or end post, with a turn buckle or other equivalent provision for adjustment. Truss rods may be eliminated in any line of fence where there is a continuous center rail.
- (3). **Swing Gates: Provide a 4' wide and a 12' wide swing gate,** Gates shall be welded steel frame of 1.9" OD, 2.27 lbs. per foot, braced and trussed a minimum of every 8 feet. Fabric: 9 gauge, Class 2 galvanized coating (2.0 oz./ft<sup>2</sup>), (2" mesh) chain link attached with 9 gauge steel, 1.8 oz. galvanized coated tie wire spaced 15" on center.
- Gate Frames shall conform to ASTM Specifications F1043 or F1083, or a combination thereof, and shall match that selected for any adjoining fence framework. Welded joints shall be coated in accordance with Practice A780
- Gate Hinges—Hinges shall be structurally capable of supporting the gate leaf and allow the gate to open and close without binding. The hinges shall be so designed to permit the gate to swing a full 180°.
- Single Gate Latch —Gate latch shall be capable of retaining the gate in a closed position and shall have provision for a padlock.
- Double Gate Latch —Gate latch shall be a drop rod or plunger bar arranged to engage the gate stop. Locking devices shall be constructed so that the center drop rod or plunger bar cannot be raised when the gate is locked. The latching devices shall have provision for a padlock.
- Gate Stops—Gate stops shall be provided for all double gates.
- Keepers shall be provided for each gate leaf over 5 ft.
- (4). **Tension or Stretcher Bars:** Hot-dip galvanized steel with a minimum length 2 inches less than the full height of fabric, a minimum cross section of ¼ inch x ¾ inch, and Class 2 galvanized coating (2.0 oz. /ft<sup>2</sup>) in accordance with ASTM F-626. Provide bars for each gate and end post, and two bars for each corner and pull post.
  - (5). **Tension Wire:** 7 Gauge coil spring galvanized tension wire attached to the bottom of fence fabric with 9 gauge steel hog ring spaced 24" on center.
  - (6). **Barbed Wire:** 3 strands of 12½ gauge 4 point barbs at 5" centers on a 45 degree pressed steel barb wire arm the top wire shall be approximately 12" horizontally from the fence line with the other wires spaced uniformly. The barbed wire arm shall be of sufficient strength to withstand a weight of 250 pounds applied at the outer strand of barbed wire. Barbed Wire shall be pulled tight to remove all sag. Firmly install in the slots of extension arms, and secure it to a terminal post utilizing terminal post band arms or brace bands or other suitable devices.
  - (7). **Tension Bands:** fabricated of galvanized steel in accordance with ASTM F-626 and shall be installed attaching the fabric and stretcher bars to all terminal posts at intervals not exceeding 15". Tension bands shall be formed from flat or beveled steel and shall have a minimum thickness after galvanizing of 0.078 inch and minimum width of 3/4 inch for posts up to 4" dia.
  - (8). **Tie wire:** 9 gauge steel with 1.8 oz. galvanized coating tie wire shall be spaced 15" on center for line posts and 24" on center for rails.
  - (9). **Post footing:** Posts shall be set in concrete with a 28 day compressive strength of at least 4,000 psi conforming to ASTM C94, "Standard Specification for Ready-Mixed Concrete" or ASTM C387, "Standard Specification for Packaged, Dry, Combined Materials for Mortar and Concrete".
- j). Outside lighting with photo cell control.

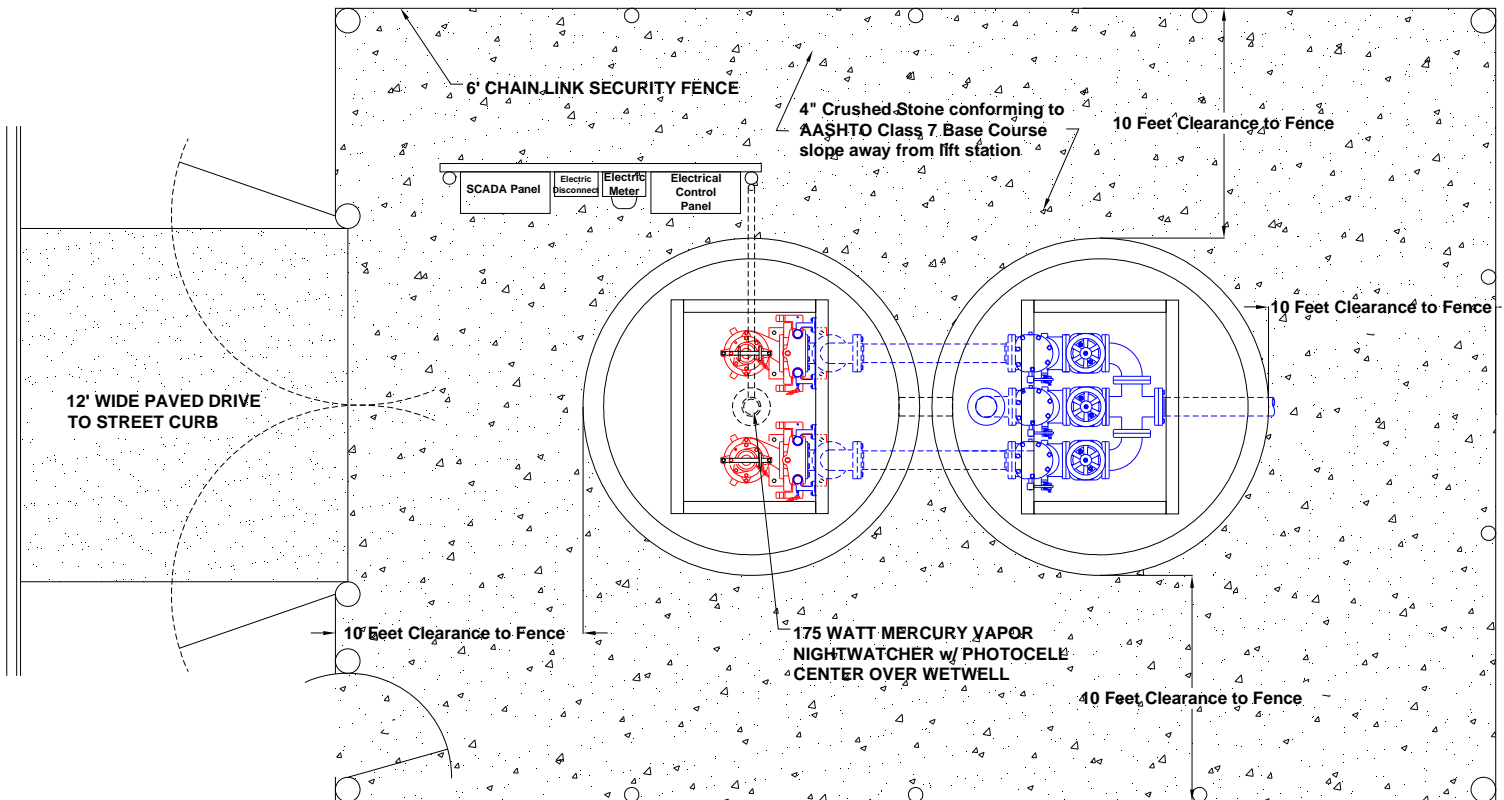
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- k). Stainless steel lifting chain and guide rails.
- l). Alarm light to be visible above security fencing.
- m). Public notice sign (12" X 18") stating "If alarm sounds or light is flashing, please call 743-1428," to be installed on outside of security fence for visibility of public.
- n). Pump hoist with permanently mounted base plate.
- o). Guard rails installed around openings of wet wells that conform to OSHA – 29 Code of Federal Regulations, Subpart M, Fall Protection, 1926.502(k). The top edge height of top rails must be 42 inches plus or minus 3 inches. Midrails must be installed at a height midway between the top edge of the guardrail system. The guardrail system must be capable of withstanding a force of at least 200 pounds applied within 2 inches of the top edge in any outward or downward direction. When the 200 pound test is applied in a downward direction, the top edge of the guardrail must not deflect to a height less than 39 inches
- p). Automatic locking wet well hatch large enough to accommodate pump removal without encumbrance.
- q). A separate electrical conduit shall be installed for each pump power cord and large enough in diameter to easily remove and replace power supply cord. Electrical conduits shall be sealed inside control panel to prevent corrosion caused by sewer gas, (No expanding foam may be used).
- r). Continuous ventilation of 12 complete air changes per hour. Explosion proof pumps will be required in lieu of ventilation.
- s). Main disconnect prior to control panel.
- t). 110 V outlets on side of control panel.
- u). Discharge valves to be located in a covered valve vault large enough for maintenance/repair and equipped with a drain.
- v). Liquid Filled Discharge pressure gauges on each discharge line (of proper range) complete with isolation valves for ease of cleaning/repair.
- w). 3-phase power operation, if available from Power Company.
- x). Landscaping and finished grading shall be designed to prevent storm run-off from collecting at the lift station.
- y). Lift Stations shall conform to the standard detail below.

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Typical Lift Station Detail



**Typical Lift Station Plan View**

**44. WASTEWATER FORCE MAINS**

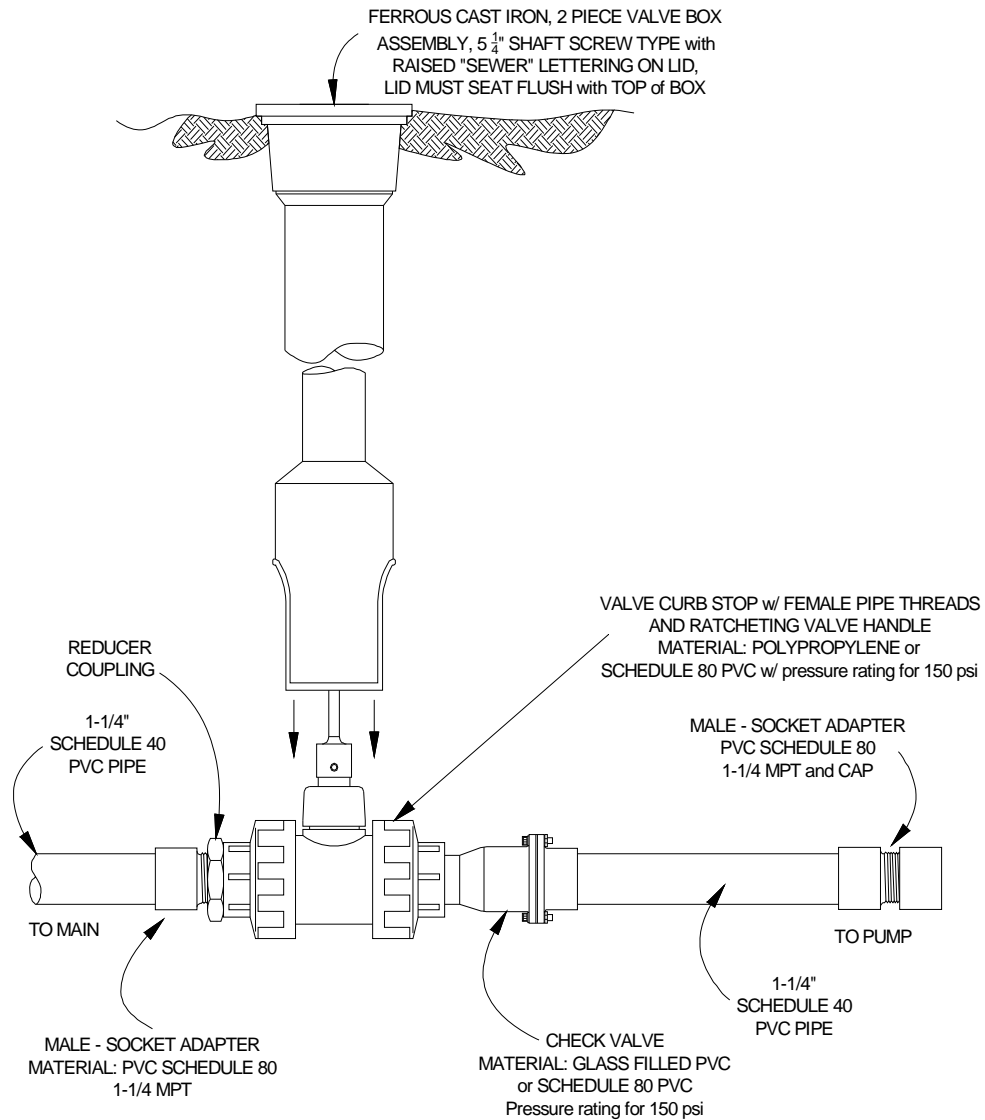
- 44.1 All requirements of Sections 1.0 through 32.0 of these specifications shall apply to sewer force mains.
- 44.2 All requirements for Tracer Wire and Marking Tape of Section 11 of these specifications shall apply to sewer force mains.
- 44.3 Force mains shall be sized at design pumping rates for a cleansing velocity of 2 feet per second. Minimum force main diameter shall be four inches.
- 44.4 Sewerline Sign Markers shall be installed every 1000 feet and at highway crossings, creek crossings, and changes in direction, railroad crossings or other areas of concern. Signs shall conform to detail in Section 11.9. Sign Markers shall not be disturbed during construction.
- 44.5 Automatic (ARI, D-025, or better) reinforced nylon, air/vacuum relief valves designed for use with sewage shall be installed at high points on the force main. Air/Vacuum Relief valve assemblies shall include carbon filters, inlet and outlet ports for back flushing, and stainless steel isolating valves to facilitate inspection and repair and be installed in a valve box of adequate size to accommodate maintenance and repair.

**45. LOW PRESSURE GRINDER PUMP SYSTEMS**

- 45.1 All requirements of Section 44, WASTEWATER FORCE MAINS shall apply for Low pressure Grinder Pumps.
- 45.2 Service Laterals for Low Pressure Grinder Pump Systems shall be installed as follows:
  - a). Service Pipe shall conform to ASTM D 1785, Schedule 40 PVC - PVC1120 or 1220 or 2120, 1¼" Dia.
  - b). Service line pipe shall be bedded in accordance with Section 10. of these specifications

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- c). Polypropylene or Schedule 80 curb stop valves shall be installed between 1' to 2' below final grade at the property line to each Building and shall be pressure-tight in both directions. The valve handle shall include a ratcheting feature to prevent breaking from over-torquing. EPDM or Buna-N O-rings shall be used to provide a redundant, watertight seal on the stem. A spherical, PVC ball shall be supported in molded, polyethylene seats to provide watertight seals in either direction, as well as maximum flow capacity and ease of operation. Valves shall be designed to withstand a working pressure of 150 psi minimum.
- d). Check Valves: Check valves shall be injection-molded from non-corroding, glass-fiber-reinforced PVC, Schedule 80 PVC, or brass for durability. The check valve flapper shall include a non-fouling, integral hinge. The check valve will provide a full-ported passageway and shall introduce a friction loss of less than 6 inches of water at maximum rated flow. The hinge shall be an integral part of the flapper assembly, providing a maximum degree of freedom to ensure seating at low back pressure. All internal metallic internal parts shall be stainless steel. No spring loaded check valves shall be permitted.
- e). Ferrous Cast Iron Valve boxes shall with a cast iron lid. The Cast Iron lid shall have "Sewer" cast into the lid with raised letters. The Cast Iron lid shall seat flush with the top of the valve box. The valve box shall extend above grade at all times until completion of building construction and final grading on each lot.



Low Pressure Grinder Pump Service Line Detail

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- 45.3 Grinder pump piping systems shall be sized to maintain flow between 2 and 3 fps velocity based on the number of pumps operating simultaneously.
- 45.4 Grinder pumps shall be E/ONE Series 1010, positive displacement pumps rated for 14gpm @ 0' TDH and 9gpm @ 138' TDH and shall consist of:

- E/ONE Grinder Pump MODEL 1010
  - 30 x 74" HDPE Basin with Stainless Steel Band Clamp
  - Model SENTRY Simplex Alarm Panel
  - 32' of Power Cable with EQD
  - Redundant Check Valve

### **46. RESTORATION AND CLEAN-UP OF JOB SITE**

- 46.1 Restoration and clean-up of all work performed in roadways, either within or outside paved areas, shall be performed in strict accordance with specifications of the regulating local authorities and governmental agencies.
- 46.2 Contractor shall provide all equipment, materials and labor to restore areas disturbed by construction to their original condition or better, as nearby as is possible in accordance with good engineering and construction practices.
- 46.3 Promptly upon completion of work and at his expense, Contractor shall remove from the site, tools, equipment, temporary structures, barricades and surplus materials. All waste and debris shall be disposed in accordance with Federal, State and local regulatory agencies having jurisdiction of same.
- 46.4 The CONTRACTOR shall remove all vegetation that has been cut or has died as a result of the work. There shall be no burning on the job site unless approved, in advance, by the ENGINEER and other agencies involved.
- 46.5 The CONTRACTOR shall restore the topography of the work site to facilitate proper drainage of all adjacent lands.
- 46.6 No work will be accepted as complete until RESTORATION AND CLEAN-UP are completed as required.
- 46.7 Guarantee as set forth in these specifications shall apply hereunder also.

### **47. GUARANTEE**

- 47.1 Contractor shall guarantee all work performed under this contract for a period of one year from the date of completion and acceptance by CWW.
- 47.2 Any defective portions of such work which is under the Contractor's jurisdiction, whether materials or workmanship, shall be replaced by Contractor at no cost to CWW during the term of the guarantee.

### **48. RECORD DRAWINGS & FINAL CONSTRUCTION COSTS**

- 48.1 During the course of work, the contractor or his engineer shall continually maintain a set of legibly marked up prints, drawings and sketches showing any changes made during the construction process. Promptly upon completion of construction, the Contractor shall submit to CWW one set of reproducible as-built record drawings. The drawings shall show the actual in-place alignment and grade of the water lines, valves, fire hydrants, and service line and/or meter boxes. Tie-down measurements shall be provided for all valves, blow-offs, service lines and/or meter boxes using back of curbs, fire hydrants and/or manholes as reference points. Each change in direction of pipe with bends shall be reflected on record drawings with tie-down measurements in a form acceptable to CWW.
- 48.2 Engineering Firms responsible for the project shall deliver to Cabot WaterWorks "as-built drawings" and AUTOCAD electronic files of the Facilities located in Arkansas State Plane South coordinates. AUTOCAD files shall include all tie-down measurements required for as-built record drawings specified above.